

台北市既有水管橋耐震評估與補強

Seismic Evaluation and Retrofit of Existing Water Pipe Bridges in Taipei

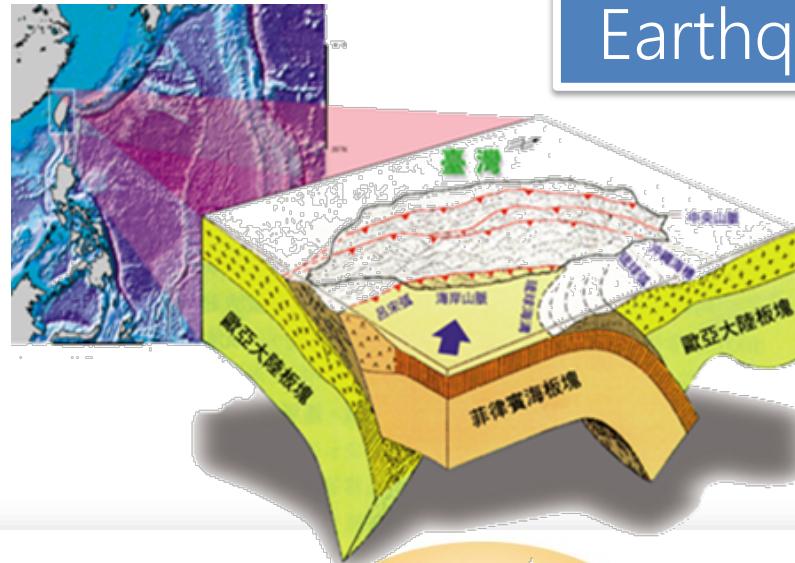
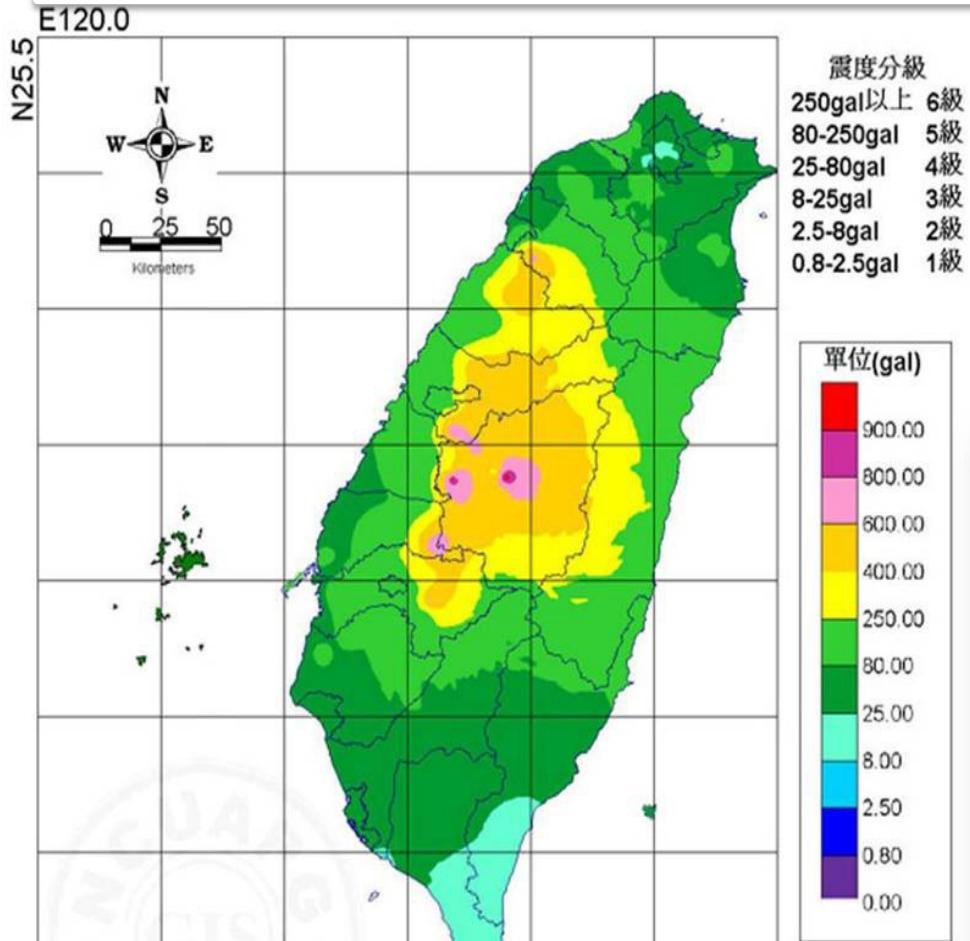
Authors: Wei-Hsiang Lee, Kuan-Hua Lien, Po-Ming Cheng
and Chii-Jang Yeh  Sinotech Engineering Consultants)

Speaker: **Po-Ming Cheng** Technical Manager

The 10th CTWWA/JWWA/WRF Water System Seismic Conference
October 18-21, 2017 Tainan, Taiwan

Motive

921 ChiChi earthquake



Earthquake



Speaker Introduction

Speaker : Po-Ming Cheng Structure P.E. ,Technical Manager

Company : Sinotech Engineering Consultant

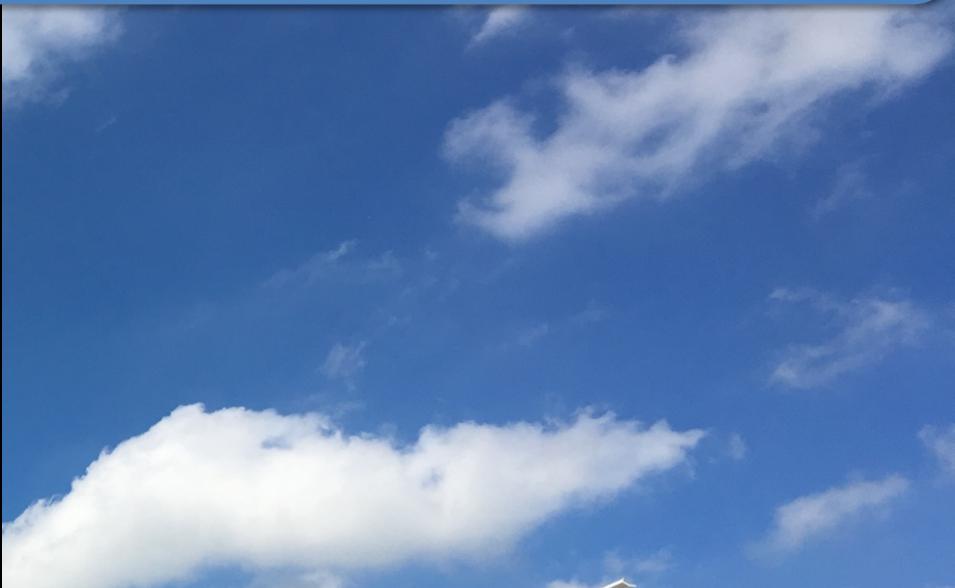
(start from 1991)

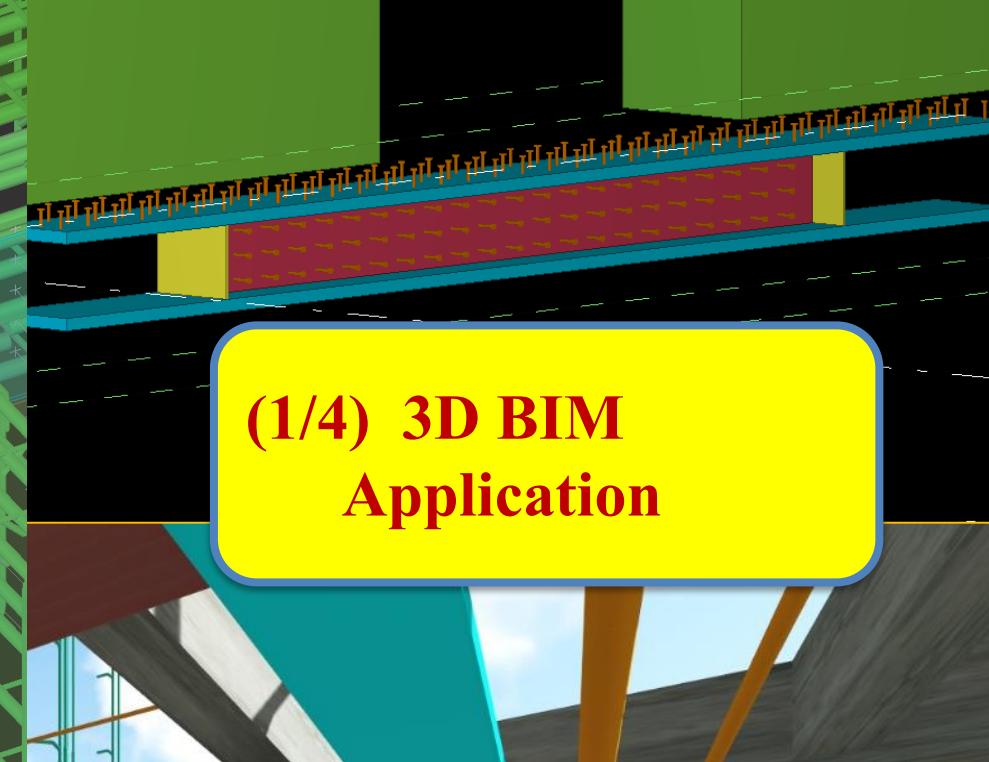
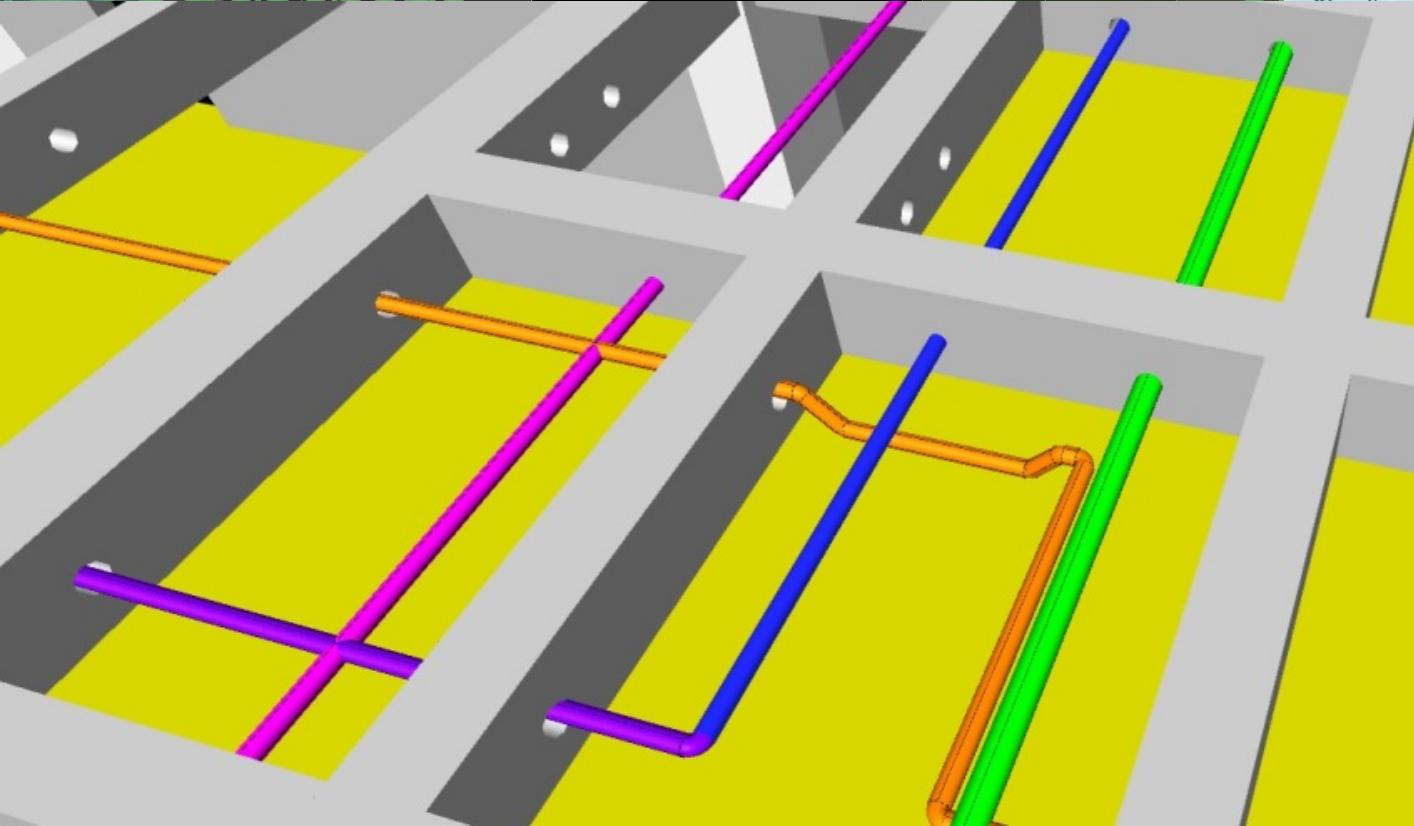
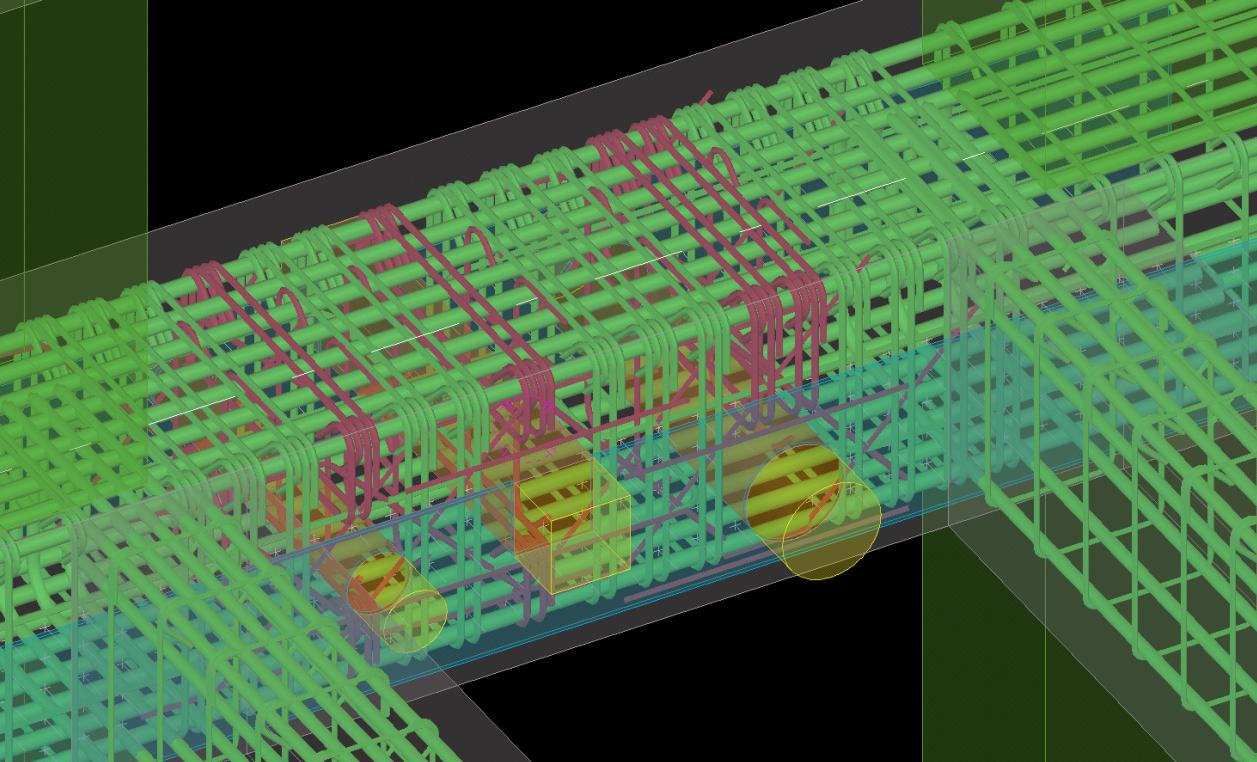
Experience:

- 1. Building Design & 3D BIM Application**
- 2. (Steel) Plant Design**
- 3. New bridge design**
- 4. Existing Bridge Evaluation & Retrofit**



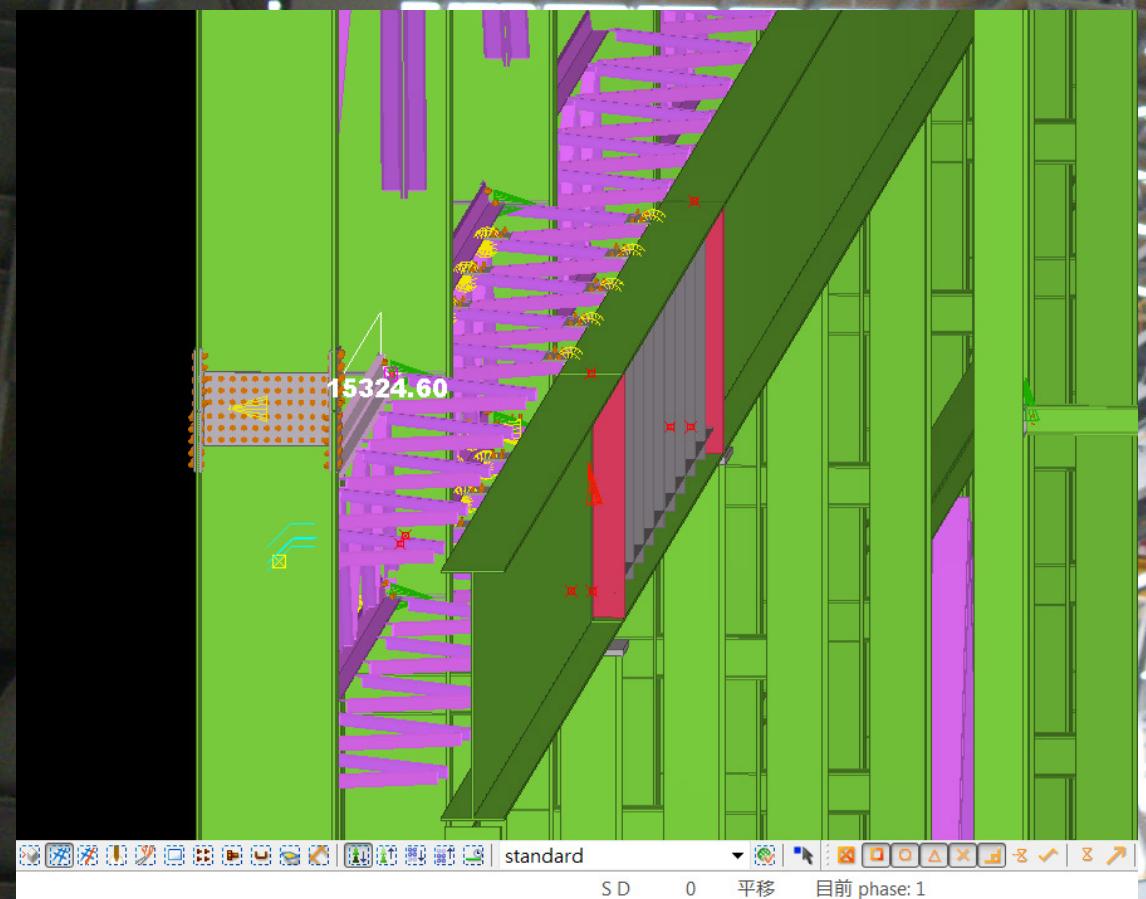
(1/4) 2017 Summer Universiade
Athletes' Village of Taipei





**(1/4) 3D BIM
Application**

(2/4) (Steel) Plant Design



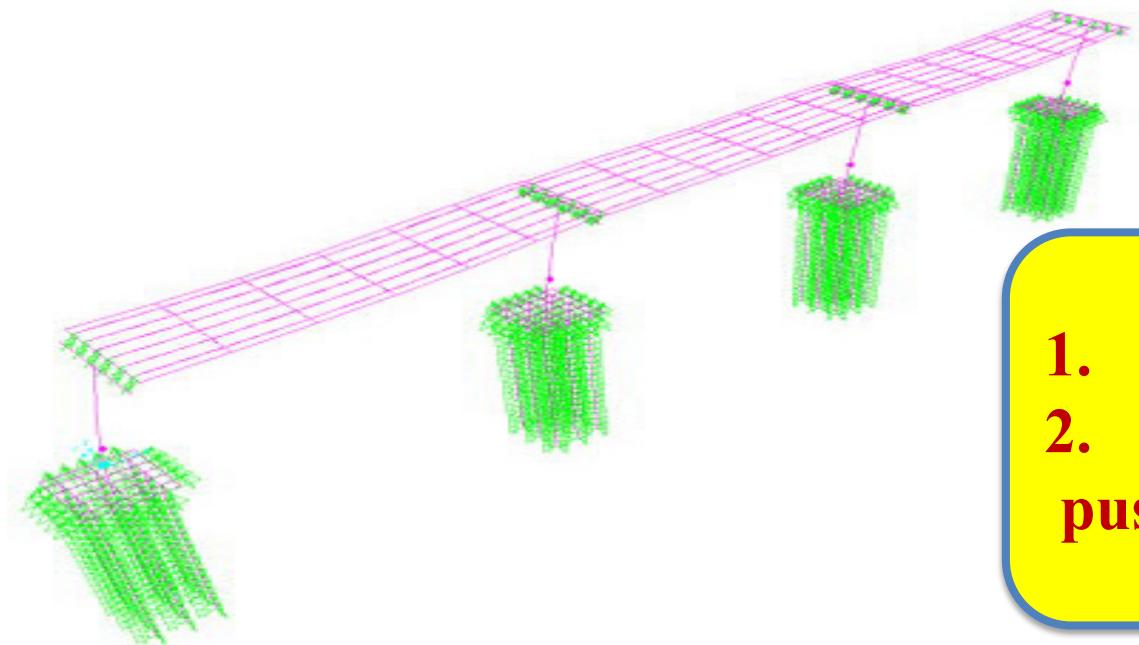
2013/05/22

台灣中山區大直橋
此為約略的地址

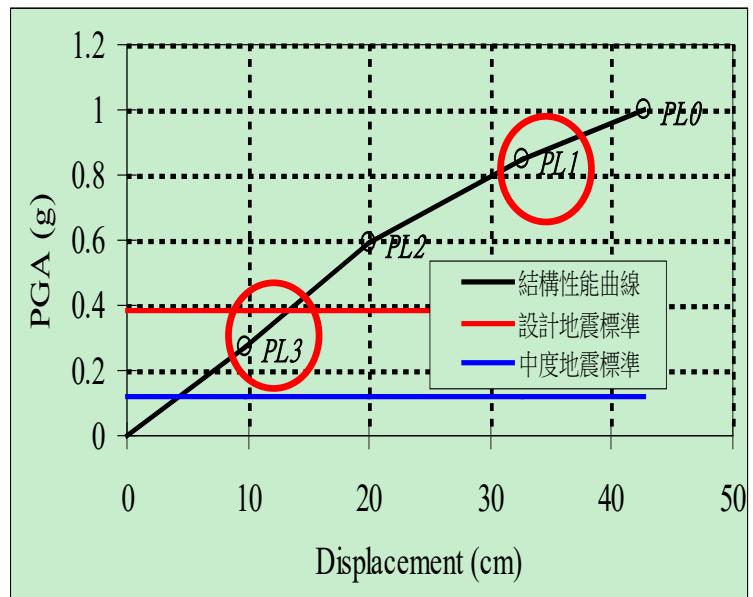


(3/4)New bridge
design

(4/4) Existing Bridge Evaluation & Retrofit



1. Project begin from 2009
2. July,2010 NCREE Drawed up push-over Draft



PL0~PL3 performance points PGA (after retrofit)			(before retrofit)
$\text{PGA}_{\text{PL0}}=$	0.997	g	0.548
$\text{PGA}_{\text{PL1}}=$	0.848	g	0.440
$\text{PGA}_{\text{PL2}}=$	0.593>0.387	g	0.304
$\text{PGA}_{\text{PL3}}=$	0.271	g	0.114

Existing Bridge Evaluation & Retrofit(4A/4)

$$V = K_h \cdot W = Z S I C_0 \cdot W = 0.8 \times 1.1 \times 1.2 \times 0.15 \cdot W = 0.16W$$

1 Column Vert.	0.16W
2 Column Hori.	0.16W
3 Bearing & E.J.	0.16W
4 Foundation	0.16W

Old Code

Year
1995

$$V = \frac{ZI}{1.2\alpha_y} \left(\frac{C}{F_u}\right)_m W = 0.607 / 3.96 \cdot W = 0.153W$$

New Code

1 Column Vert.	0.153W
2 Column Hori.	0.607W (or Vp)
3 Bearing & E.J.	0.607W (or Vp)
4 Foundation	0.607W (or Mp,Vp)

Existing Bridge Evaluation & Retrofit(4B/4)

3 Bearing & E.J.

(mostly NG)

Retrofit \$ about 2-5%

2 Column Hori.

(mostly NG)

Retrofit \$ about 5%

4 Foundation

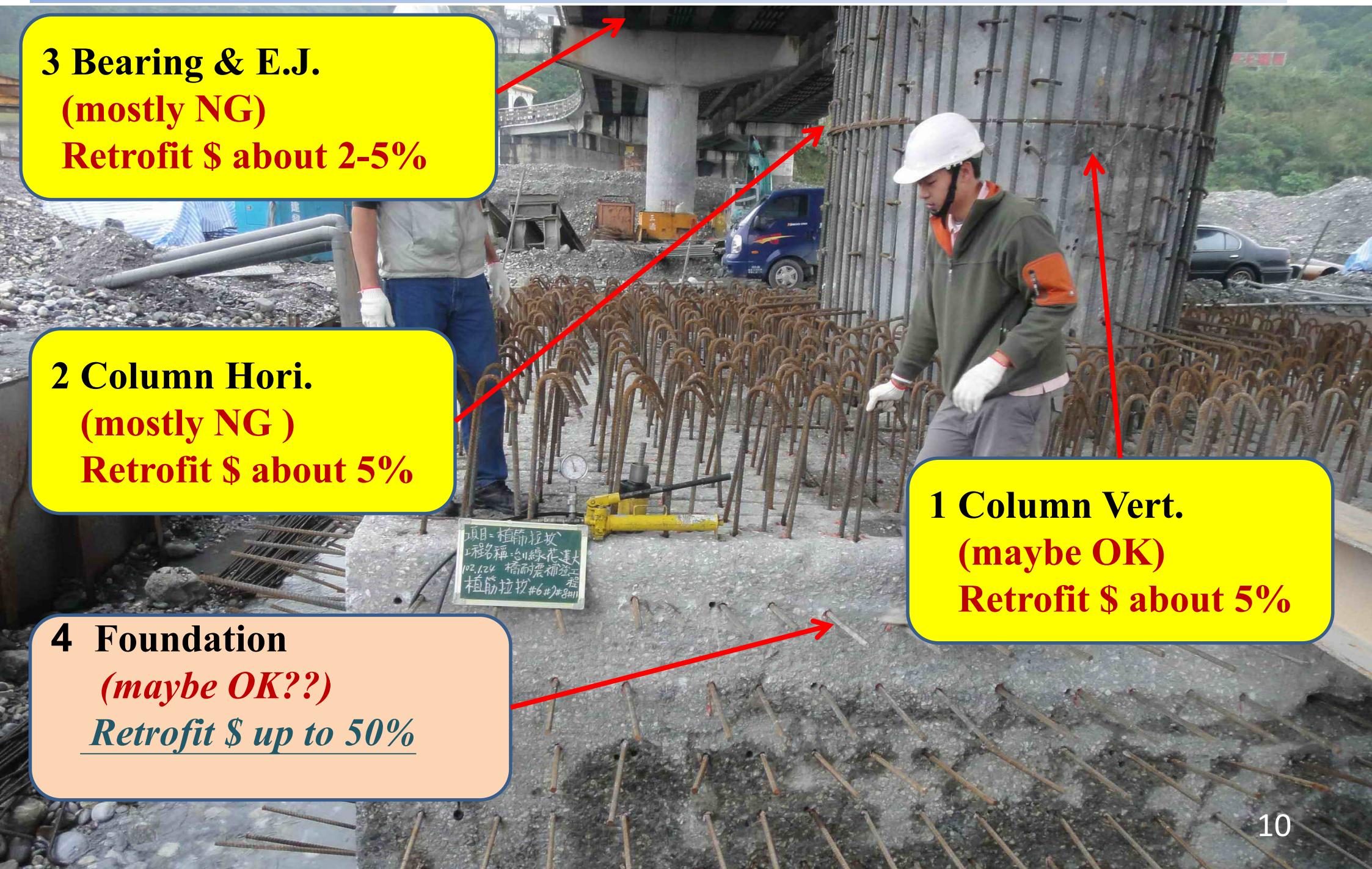
(maybe OK??)

Retrofit \$ up to 50%

1 Column Vert.

(maybe OK)

Retrofit \$ about 5%

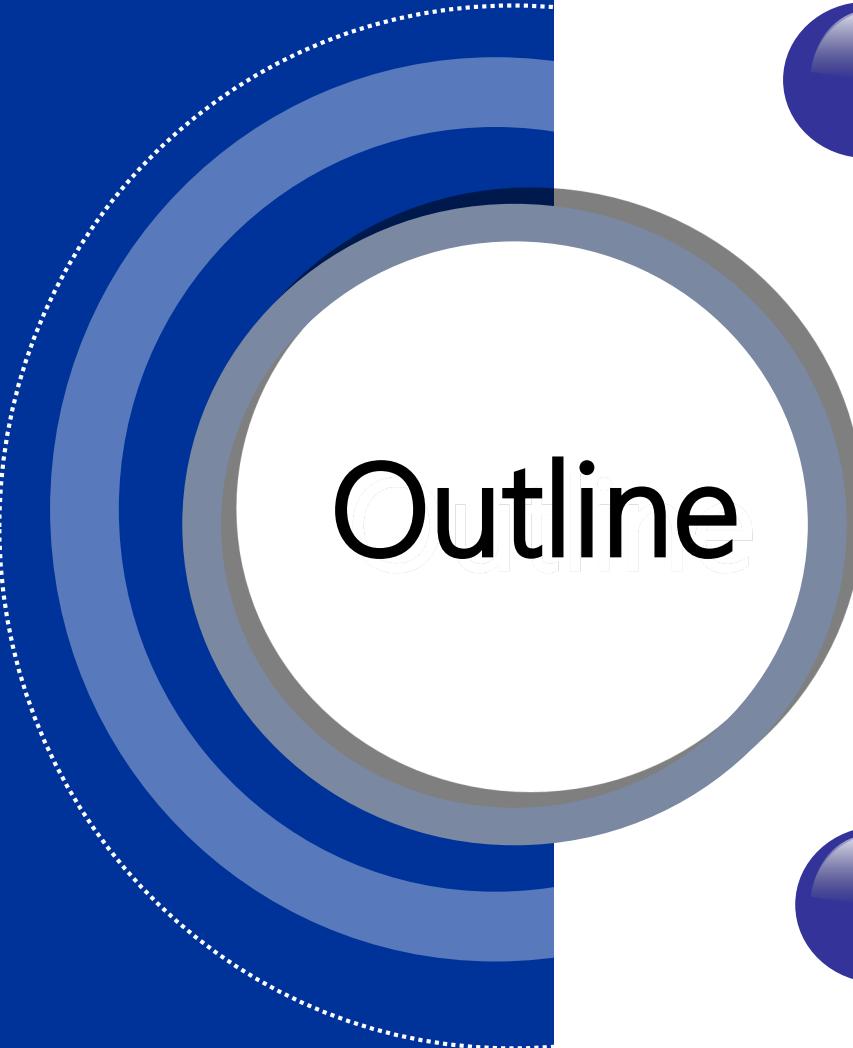


Existing Bridge Evaluation & Retrofit (4C/4)

Push-Over not on safe side ??

1. **30 % other direction not considered**
2. **Vertical E.Q. not considered**
3. **Temp. & CR&SH not considered (Continuous Bridge)**
4. **Soil Spring makes stiffness soft (Period increase)**
but Soil Spring makes E.J. safe





Outline

Introduction

Seismic Evaluation of
Water Pipe Bridge

Evaluation results
and retrofit strategy

Conclusion

01

Introduction

Location map of water pipe bridges



Water pipe bridge A



Water pipe bridge B



Water pipe bridge C



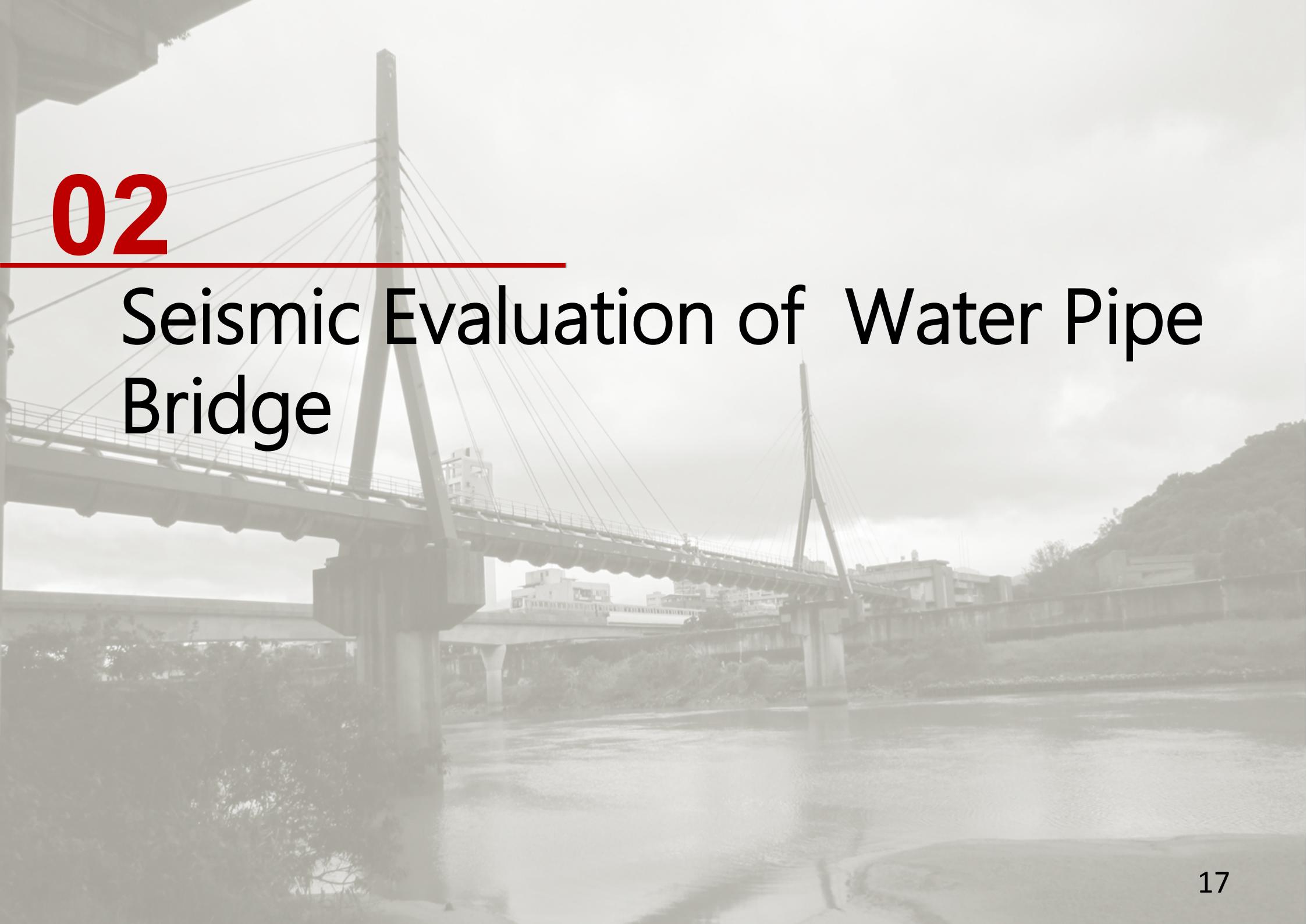
Water pipe bridge D

Basic information of water pipe bridges

Bridge name	Type (Completion year)	Span, Pipe length and diameter	Condition
Water pipe bridge A (Yuanshan)	Arch steel bridge (1988) Old Code	§ 2000mm steel pipe length :100.6 m 1 span:100.6 m	
Water pipe bridge B (Jiantan)	Cable-stayed steel bridge (1988) Old Code	§ 2000mm steel pipe length :140 m 3 spans: 34+72+34 m	
Water pipe bridge C (Yongfu)	Arch steel bridge (1987) Old Code	2- § 2400mm steel pipe Length: 360 m 7 spans: 28.2+35+3@80 steel arch+30.16+26.2 m	
Water pipe bridge D (Hsintien)	Arch steel bridge (1998) New Code	2- § 2400mm steel pipe Length: 290 m 5 spans: 25+3@80 steel arch+25 m	

Basic information of water pipe

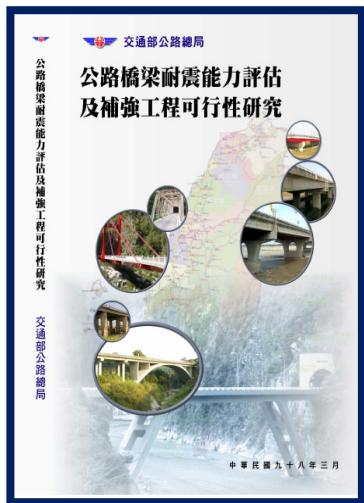
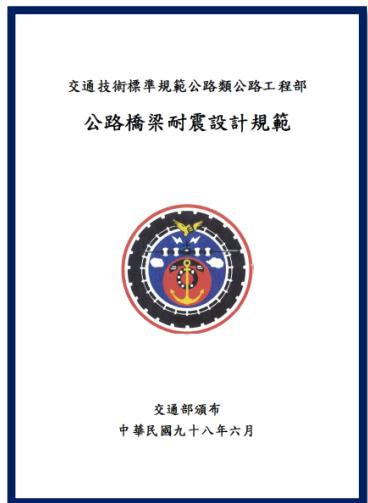
Bridge name	Diameter	Length	Unit length	Elongation	Shrinkage	Eccentricity
Water pipe bridge A (Yuanshan)	2000 mm	98 m	1600 mm	190 mm ×2	220 mm	61 mm
Water pipe bridge B (Jiantan)	2000 mm	140 m	1600 mm	190 mm ×2	220 mm	61 mm
Water pipe bridge C (Yongfu)	2400 mm	360 m×2	1400 mm 1500 mm	80 mm ×2 140 mm ×2	100 mm 160 mm	40 mm 40 mm
Water pipe bridge D (Hsintien)	2400 mm	290 m×2	1600 mm	80 mm ×2	100 mm	40 mm



02

Seismic Evaluation of Water Pipe Bridge

Standard of seismic Evaluation and Retrofit

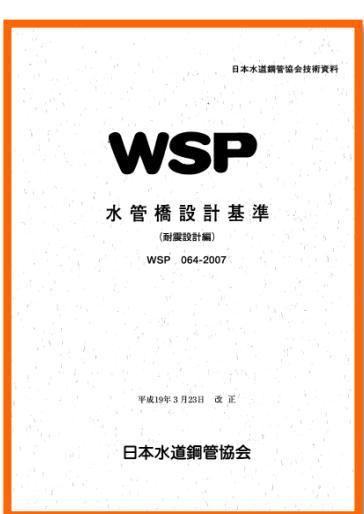
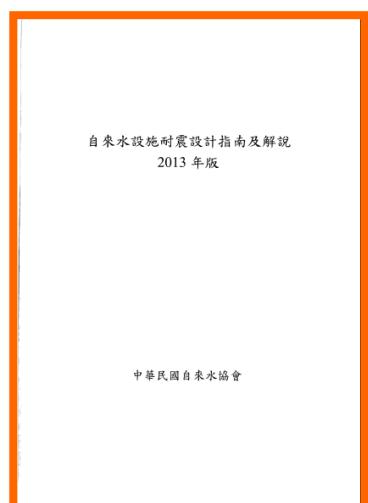


MOTC

Seismic retrofit standard: 50-year service life

Bridge objective

Normal water supply after earthquake



Retrofit strategy

- System reinforcement
- Component reinforcement
- Component increase

Seismic retrofit performance criteria & objective

■ Performance criteria (Site : Taipei 1, 2 and 3 district)

Earthquake level	Horizontal acceleration coefficient	Seismic restraint concept	Service performance	Damage grade
<u>Moderate earthquake</u>	Divided by administrative region 1/3.25 of earthquake of 475-year return period	Structure keeps elastic	Normal water supply after earthquake	<u>Slight</u>
<u>Design earthquake</u> (I=1.2) Return period:975years 50-year exceeding probability:5%	Divided by administrative region S_S^D 0.72 S_1^D 1.60、1.30、1.05	Component has plastic hinge, exerting admissible toughness capacity	Limited water supply after earthquake	<u>Repairable</u>

■ Seismic performance level

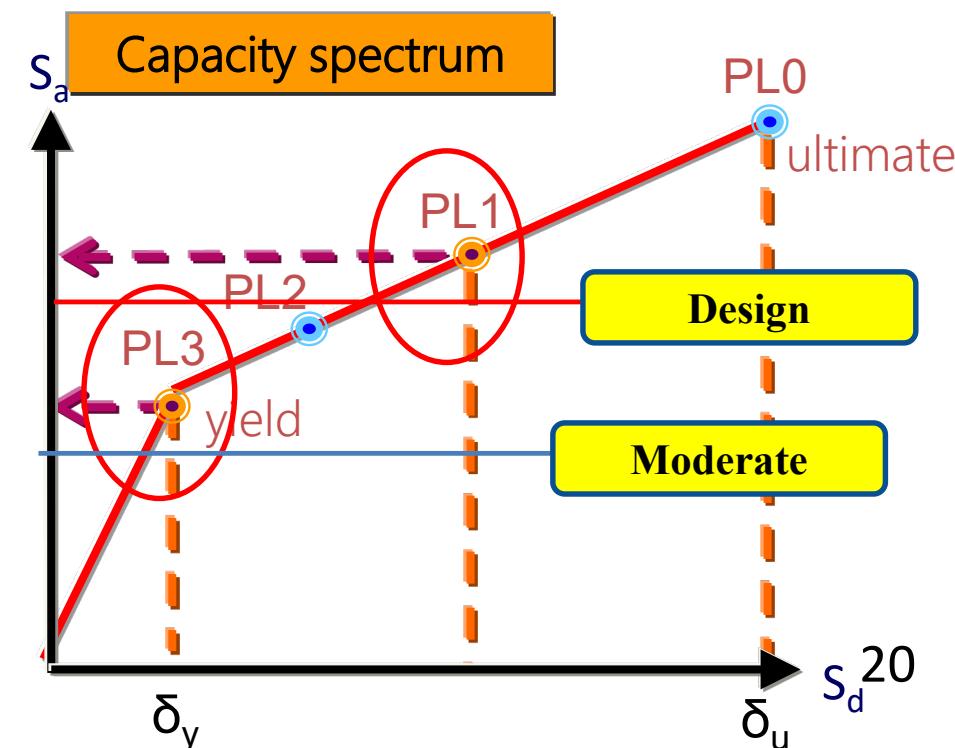
Performance level	Safety		Serviceability	Reparability	
	structure	foundation		Short period	Long period
<u>PL3</u>	Structure remains elastic	elastic	As same as prior to the earthquake	<u>none</u>	
<u>PL1(2)</u>	limited damages	elastic	Normal water supply	Partially repair or replace damaged member	<u>19</u>

Seismic retrofit performance criteria & objective

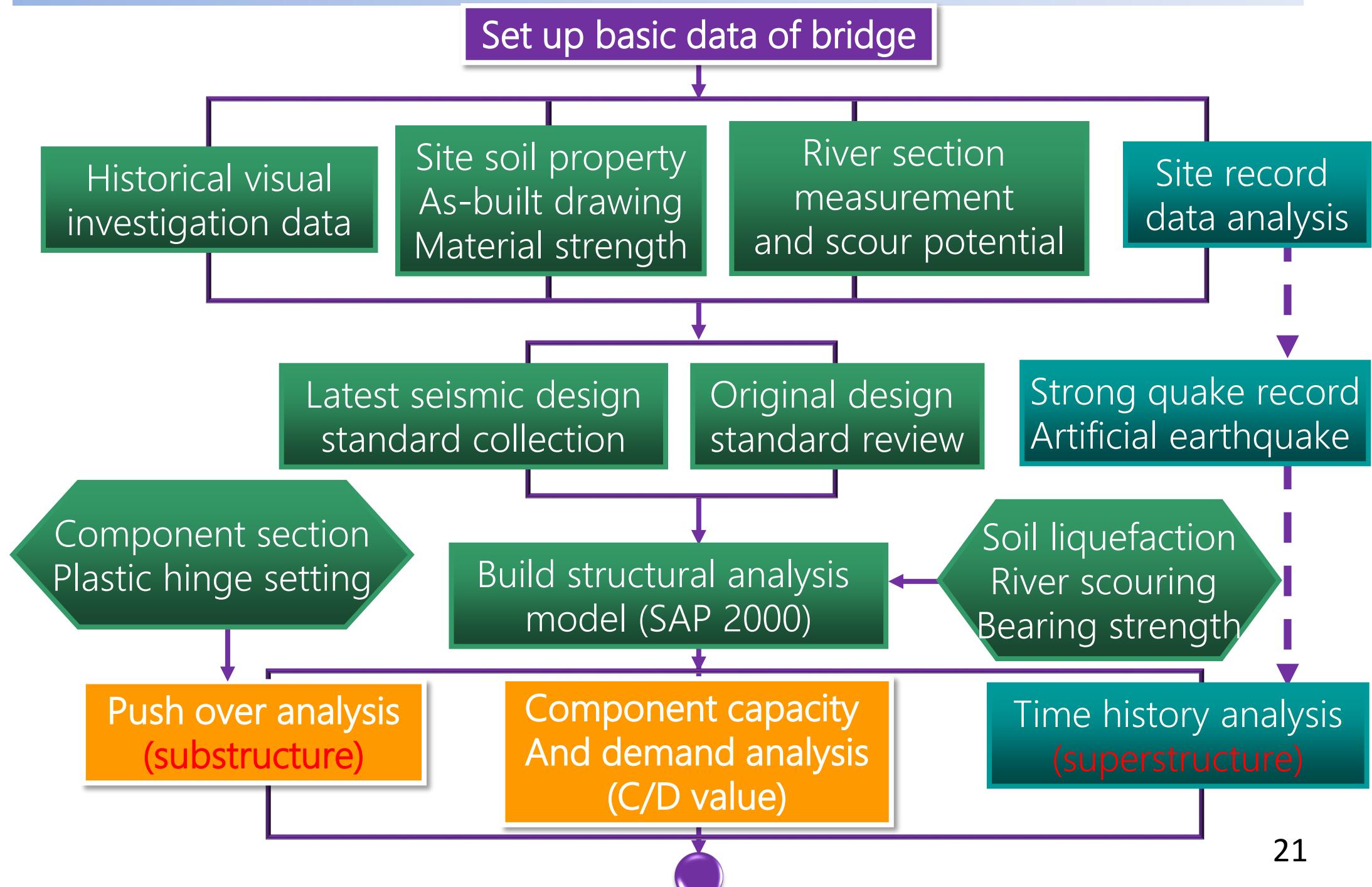
■ Seismic performance objective

Earthquake level		Design specification version					
		2000及1995 year		1987 and 41960 year		Before 1960 year	
		Ordinary	Important	Ordinary	Important	Ordinary	Important
Moderate		PL3	PL3	PL3	PL3	PL3	PL3
Design	Ordinary($I=1.0$)	PL2	-	PL1	-	PL0	
	Important($I=1.2$)	-	PL2	-	PL1		PL1

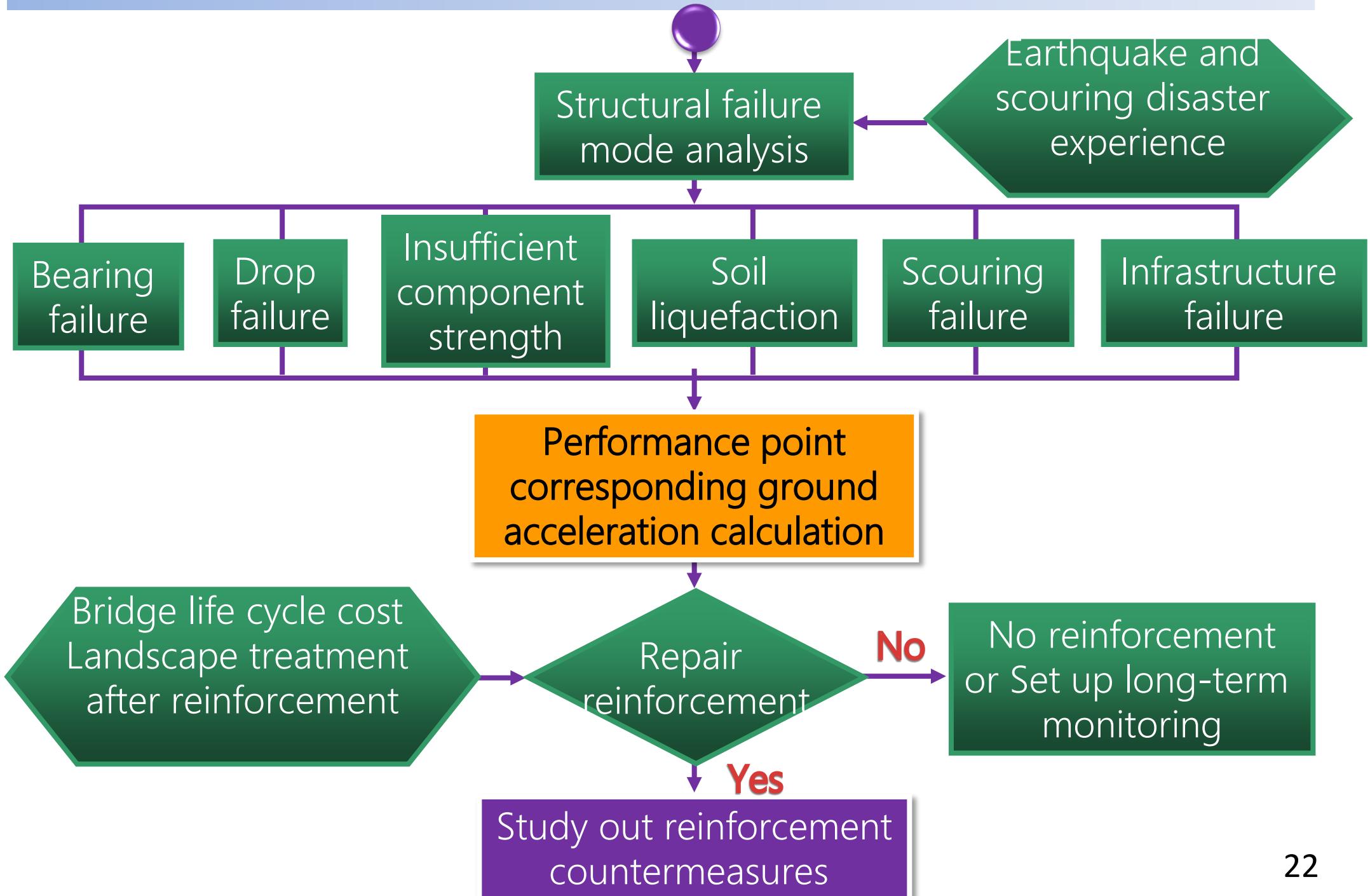
Earthquake level	Seismic performance level	Structural response condition
Moderate	PL3	Structure remains elastic
Design	PL2	Structure into inelastic and 1/4 capacity developed
Max. considered	PL1	Structure into inelastic and 2/4 capacity developed
-	PL0	Structure into inelastic and all capacity developed



Seismic evaluation of water pipe bridge



Seismic evaluation of water pipe bridge



Time history analysis for superstructure

Pipe expansion joint



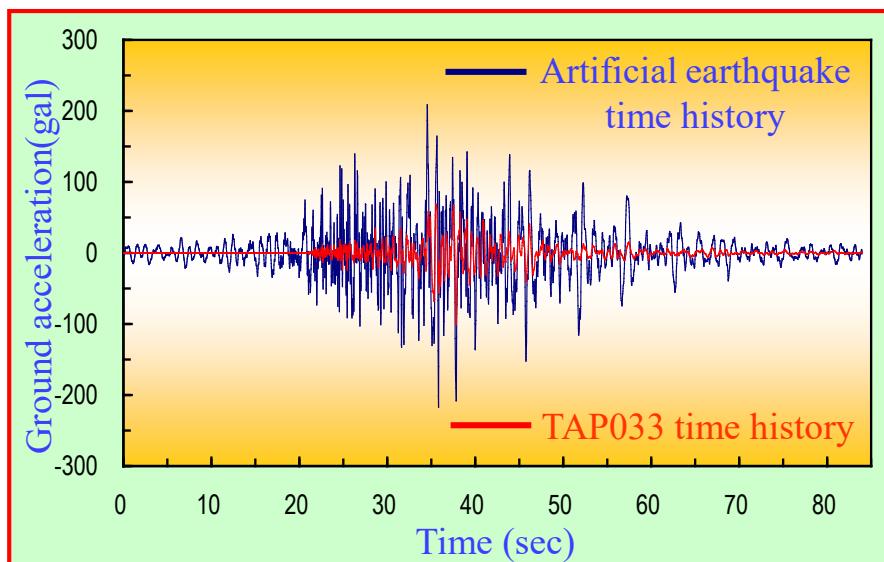
Bearing



Beam splicing



Artificial earthquake time history



Spandrel column splicing



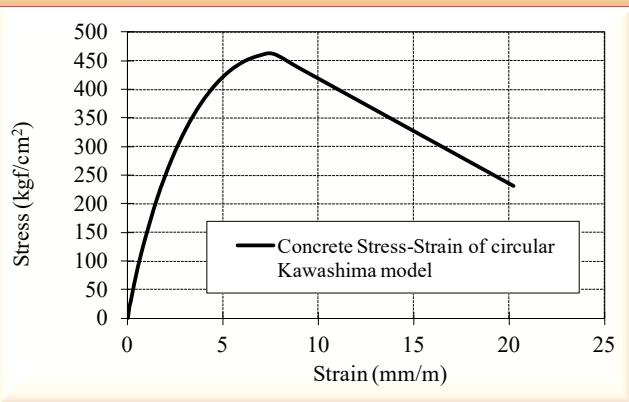
Girder splicing



*Check whether the component capacity
meet the current design code*

Pushover analysis for substructure

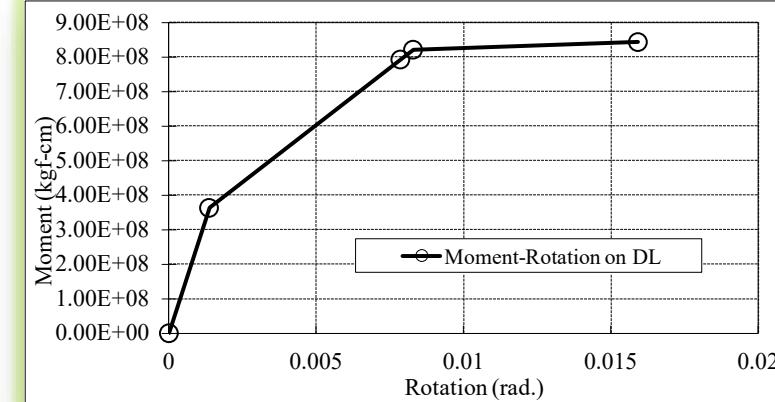
Concrete of pier
Stress-strain relation



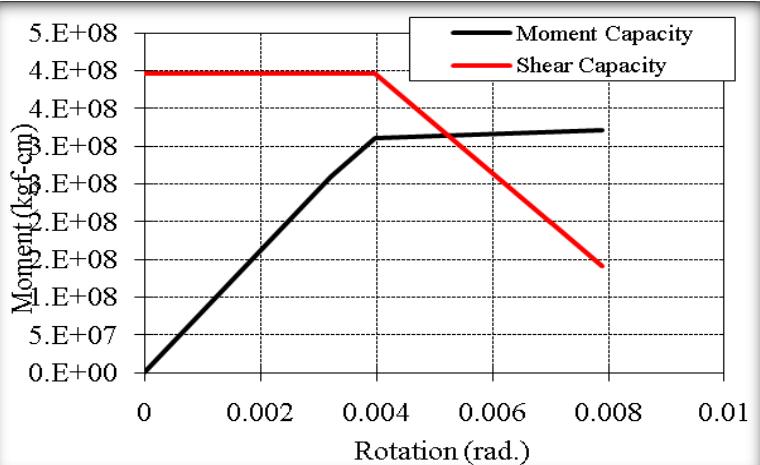
Pier moment-curvature relation



Pier moment-rotation relation

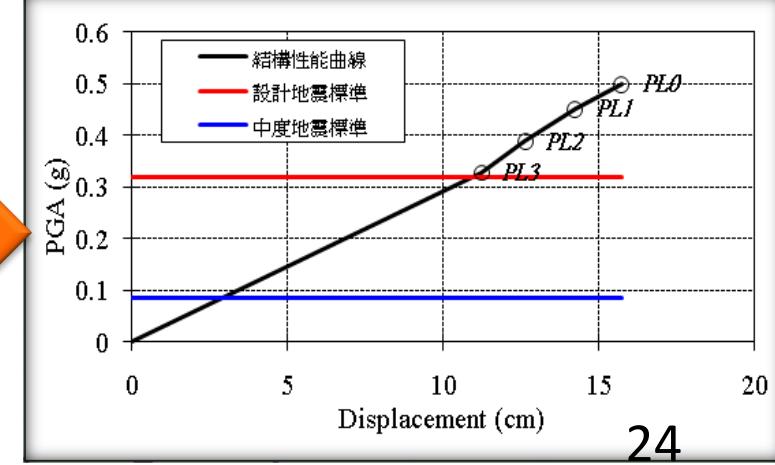


Pier
hinge curve



Push-over
Analysis

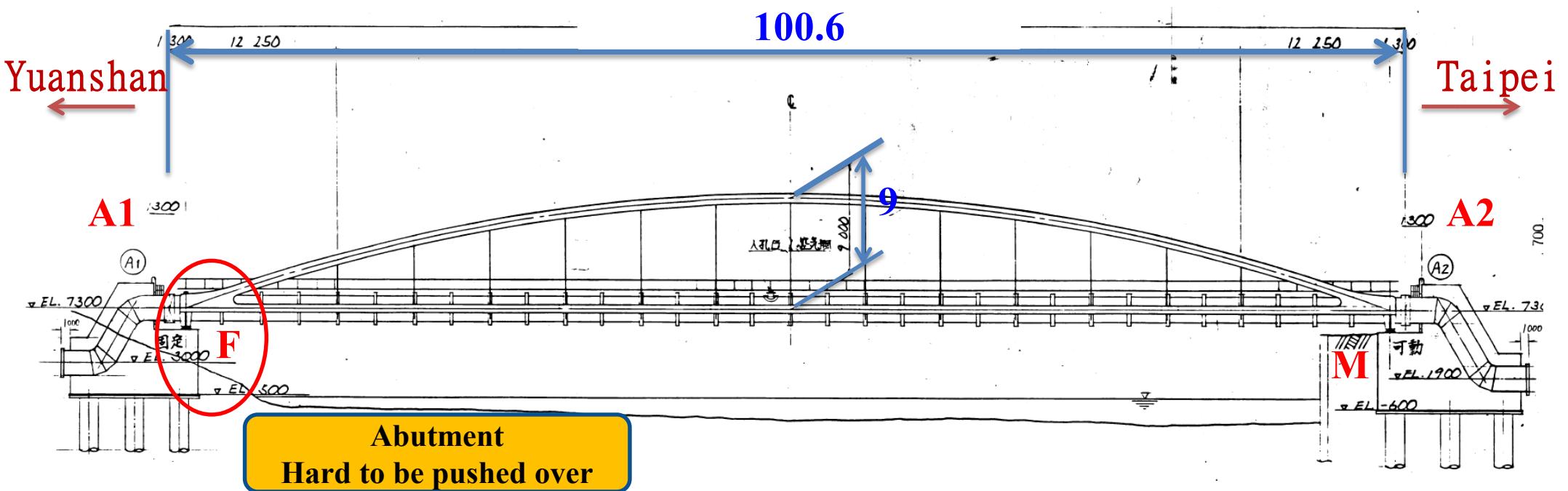
Bridge seismic
performance curve



03

Evaluation results and retrofit strategy

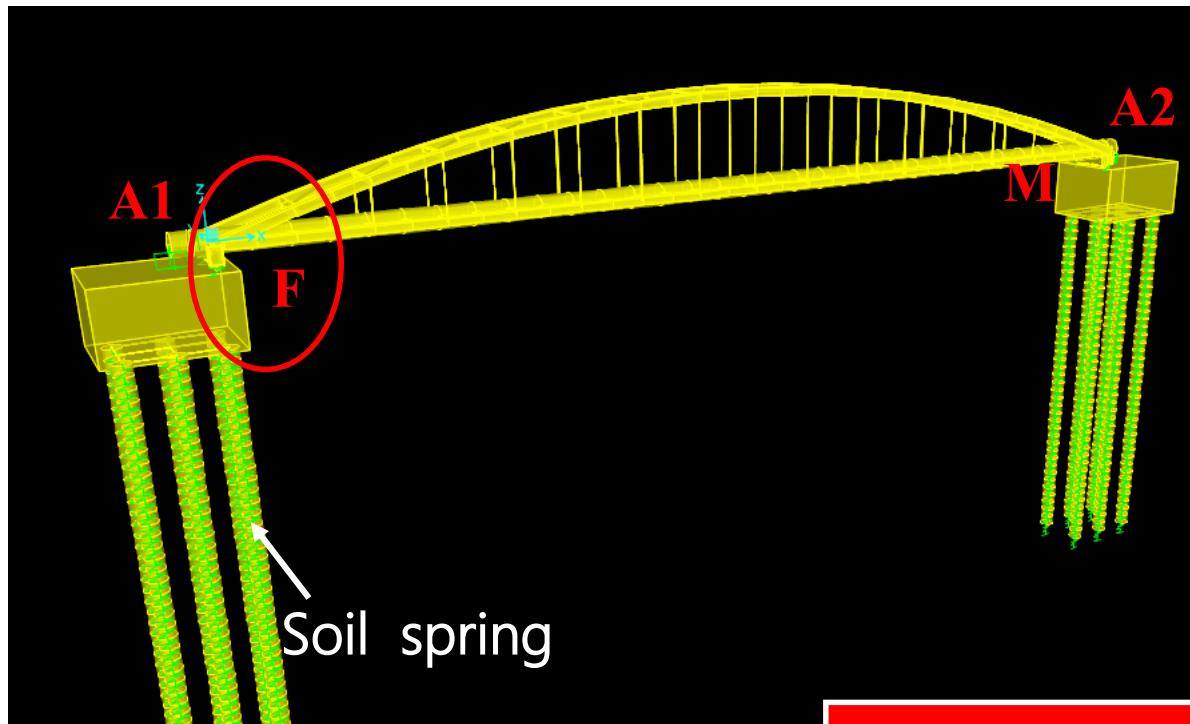
Water pipe bridge A-structure description



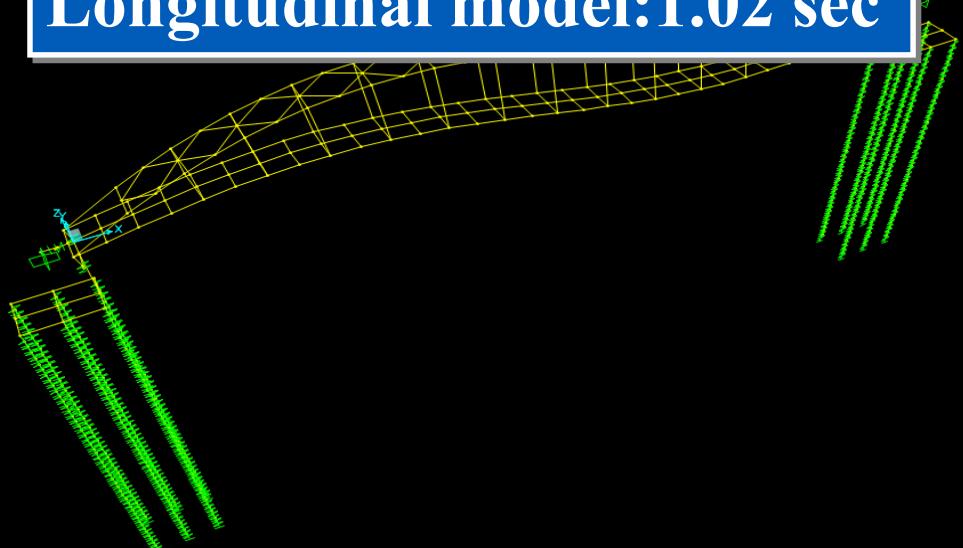
Structure description

- ❖ Single-span arch steel bridge; completion year in 1988
- ❖ Bridge length: 100.6 m; arch height: 9 m
- ❖ Steel pipe diameter: 2000 mm; two steel box girder
- ❖ Concrete: $f_c' = 240 \text{ kgf/cm}^2$; steel: A36

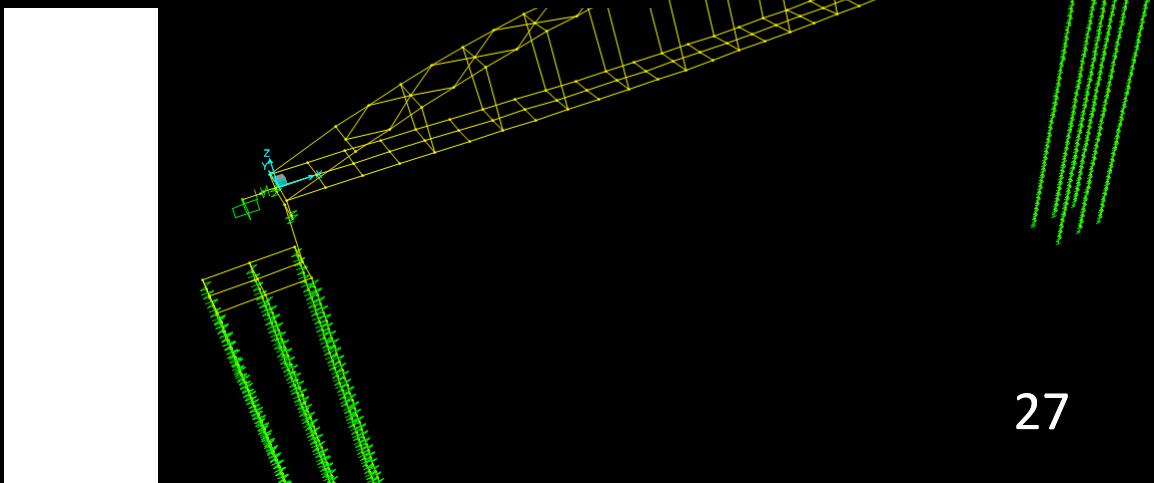
Water pipe bridge A-SAP2000 model and modal analysis



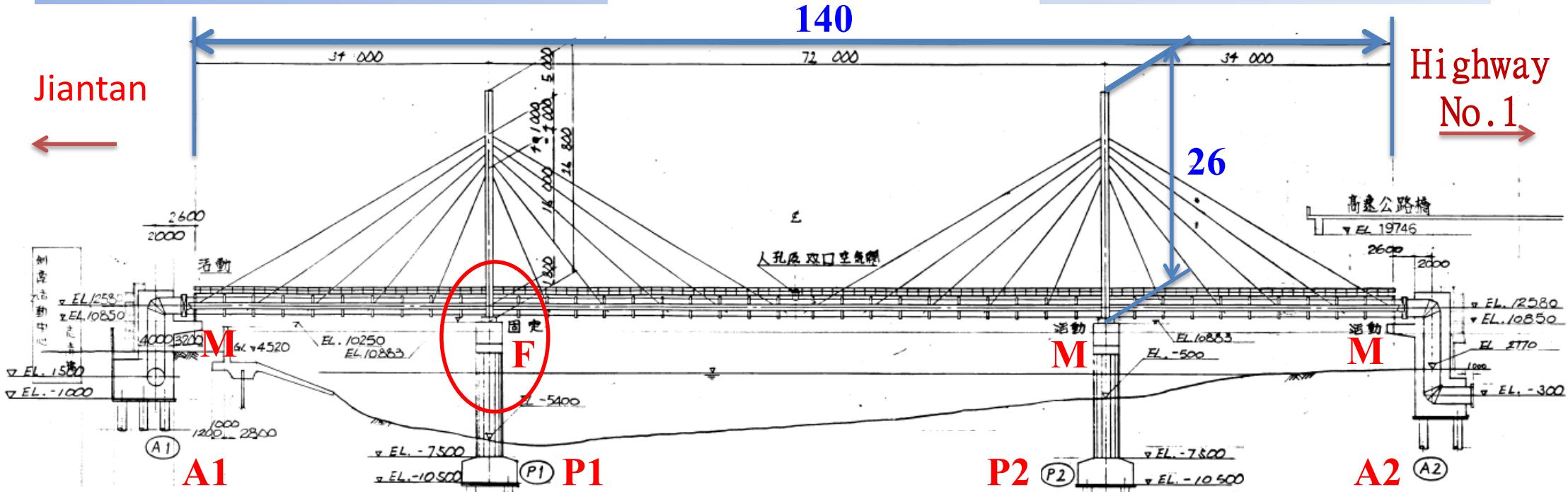
Longitudinal model:1.02 sec



Transverse model:0.52 sec



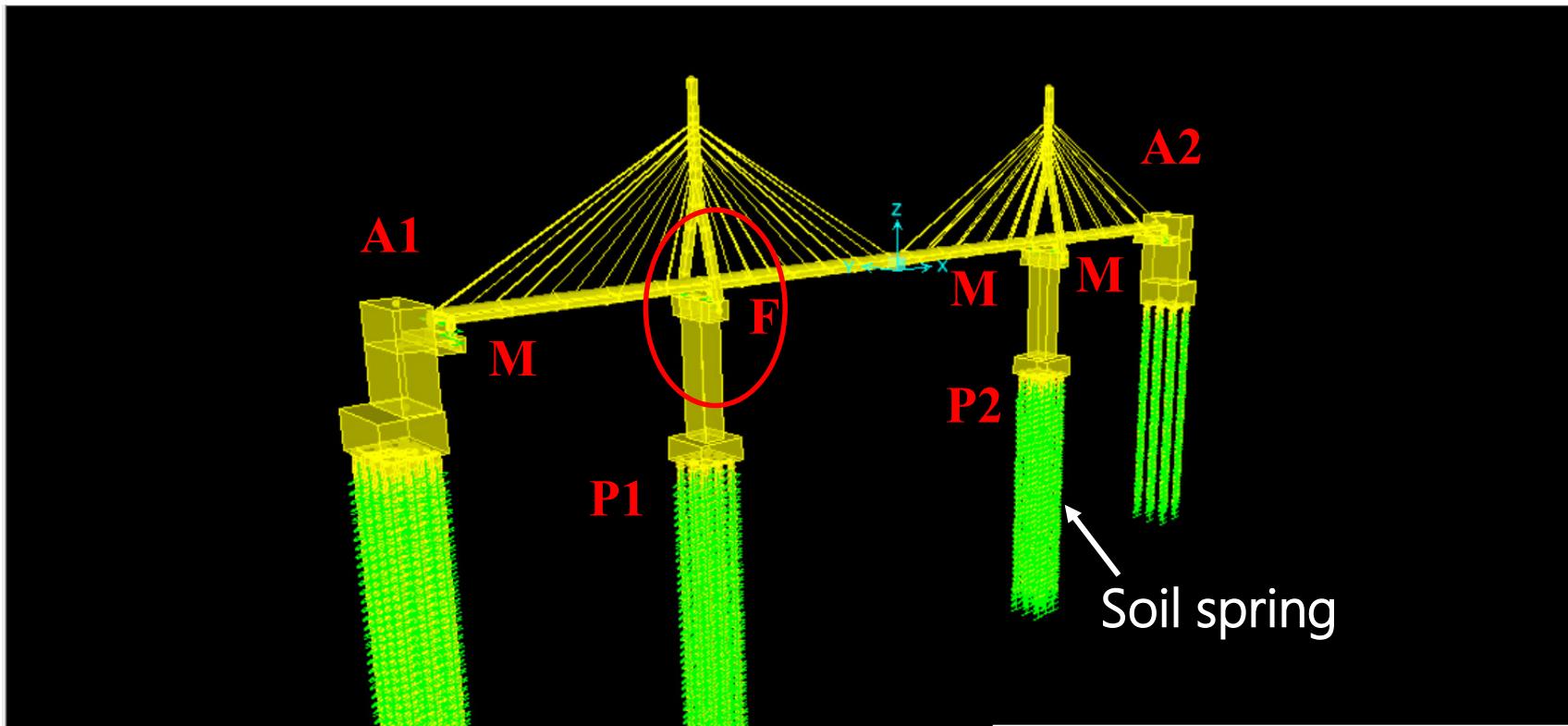
Water pipe bridge B-structure description



Structure description

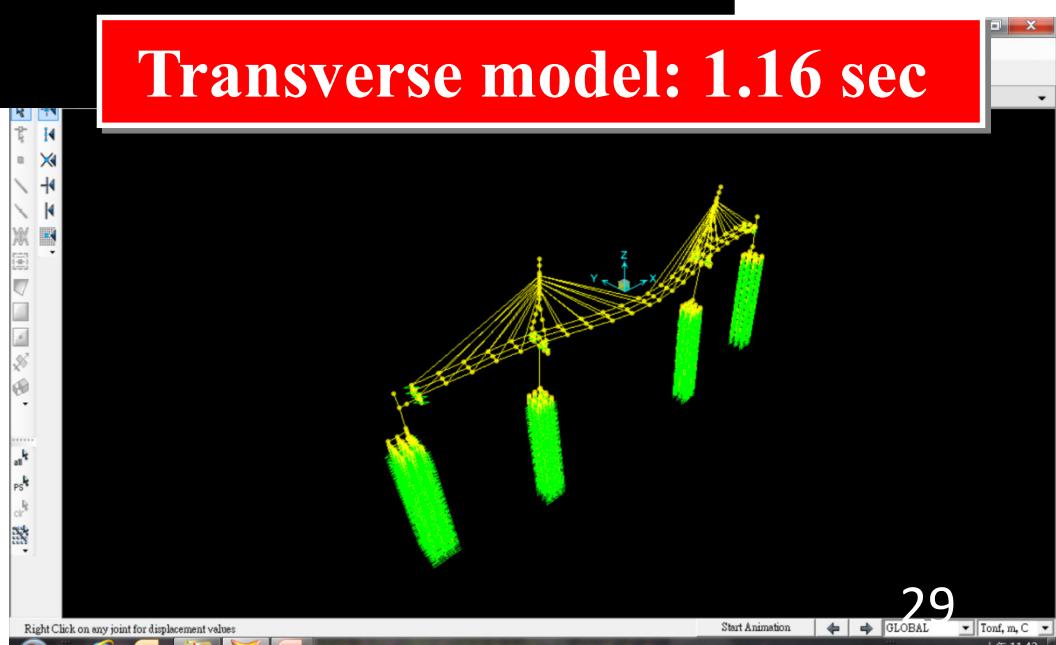
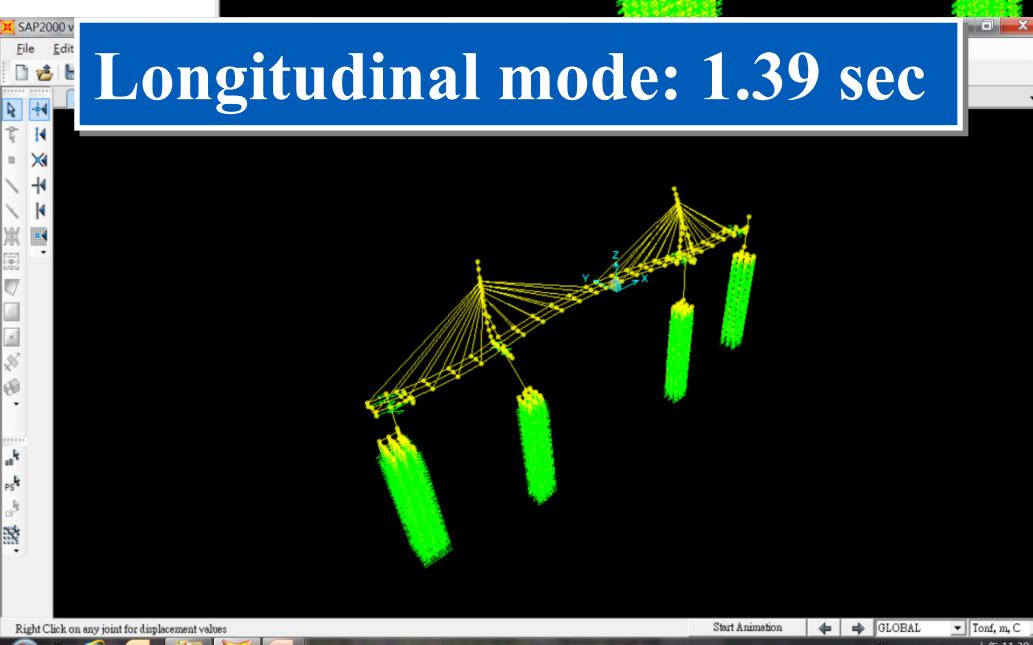
- ❖ multi-span simply supported cable-stayed steel bridge;
- completion year in 1988 Old Code
- ❖ Bridge length: 140 m; tower height: 26 m; 20 pair cables
- ❖ Steel pipe diameter: 2000mm; two steel box girder
- ❖ Concrete: $f_c' = 240 \text{ kgf/cm}^2$; steel: A36

Water pipe bridge B-SAP2000 model and modal analysis



Longitudinal mode: 1.39 sec

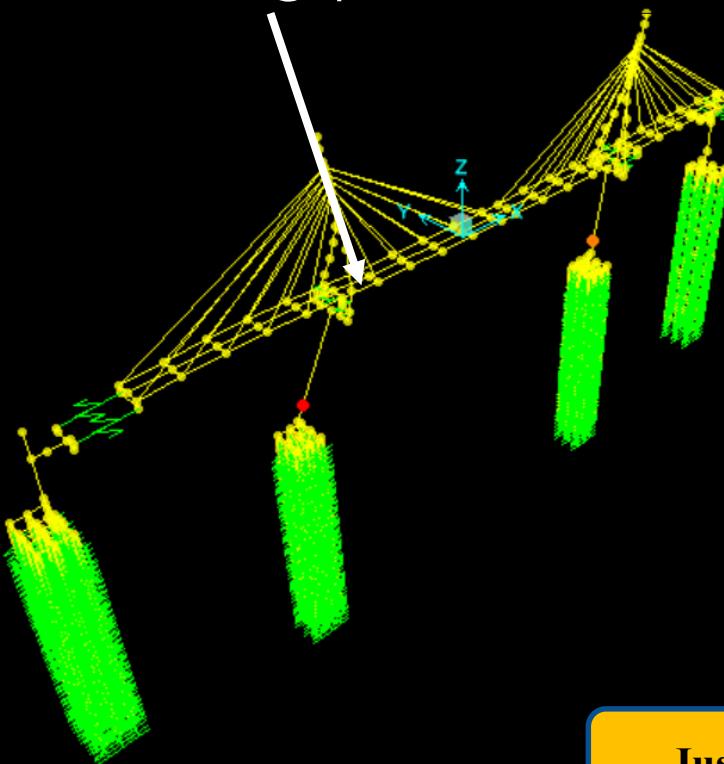
Transverse model: 1.16 sec



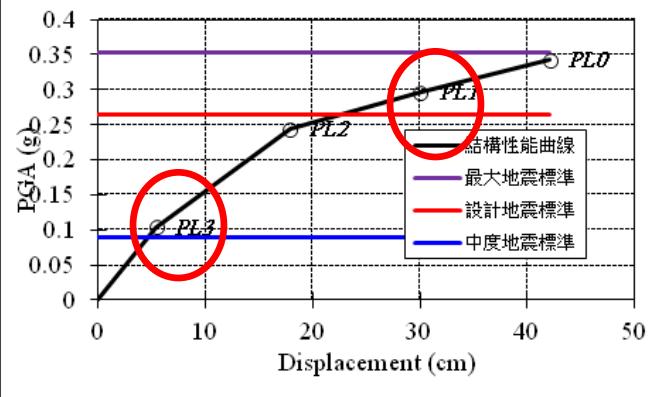
Pushover analysis(1/2)

Longitudinal

Monitoring point



Just OK

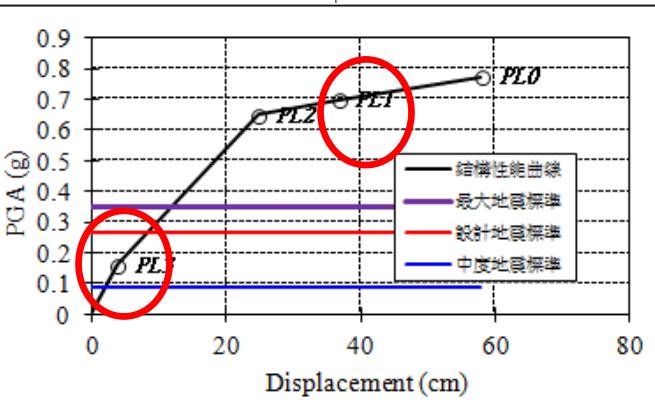
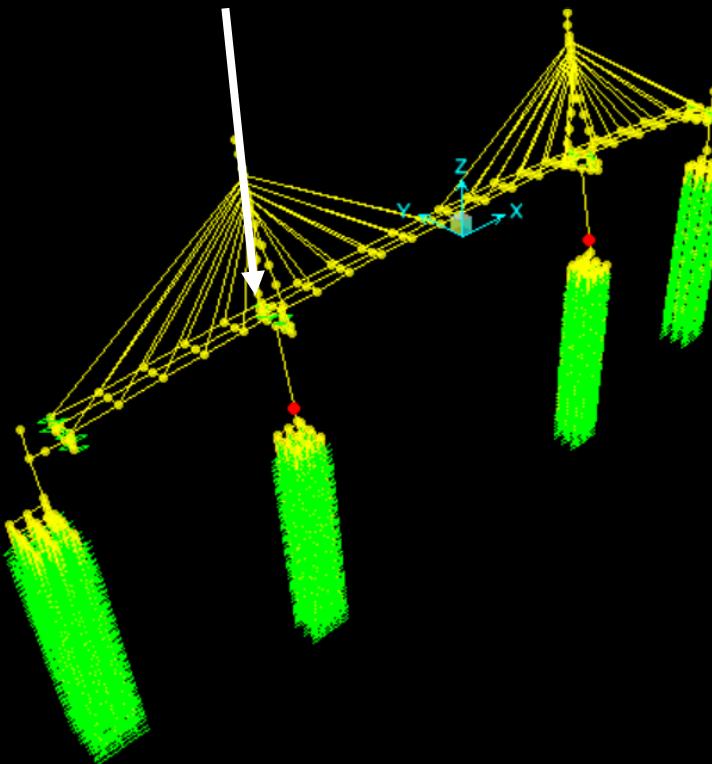


Performance objective		Analysis unit		
Earthquake level	Ground Acc. (g)	performance	PGA (g)	Seismic demand check
Moderate	0.074 g	PL3	0.104	OK
Design	0.240 g	PL1	0.296	OK ₃₀

Pushover analysis(2/2)

Transverse

Monitoring point



Performance objective	Analysis unit	Seismic demand check		
Earthquake level	Ground Acc. (g)	perform ance	PGA (g)	Seismic demand check
Moderate	0.074 g	PL3	0.160g	OK
Design	0.240 g	PL1	0.698 g	OK

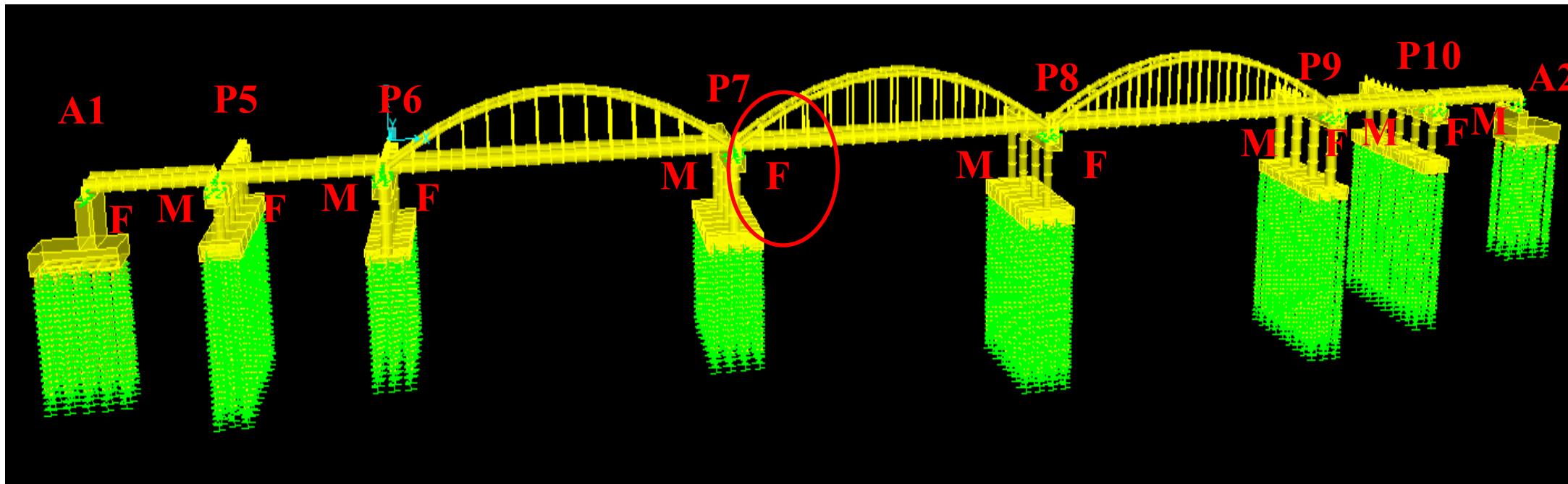
Water pipe bridge C-structure description



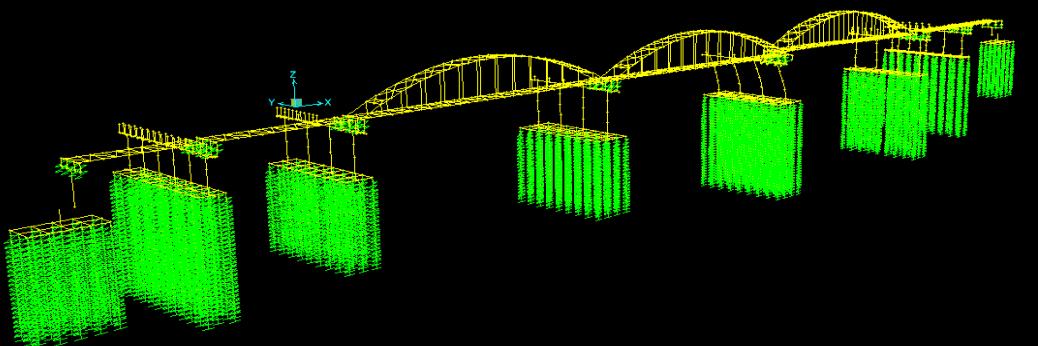
■ Structure description

- ❖ multi-span simply supported arch steel bridge; completion year in 1984 Old Code
 - ❖ Bridge length: 359.60 m; arch height: 11 m
 - ❖ Steel pipe diameter: 2400mm;
 - ❖ Concrete: $f_c' = 240 \text{ kgf/cm}^2$; steel: A36

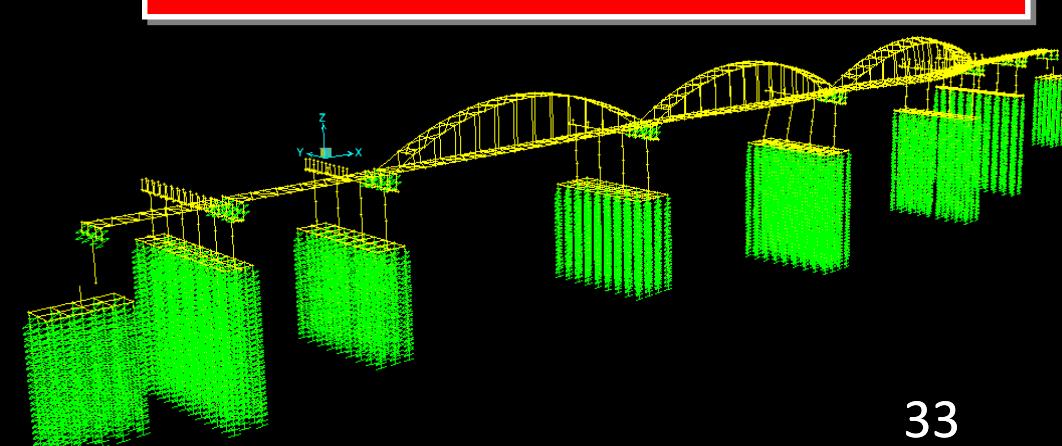
Water pipe bridge C-SAP2000 model and modal analysis



Longitudinal model: 1.55 sec



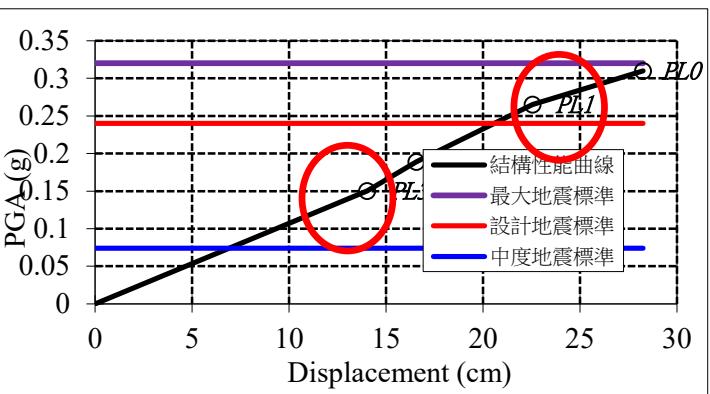
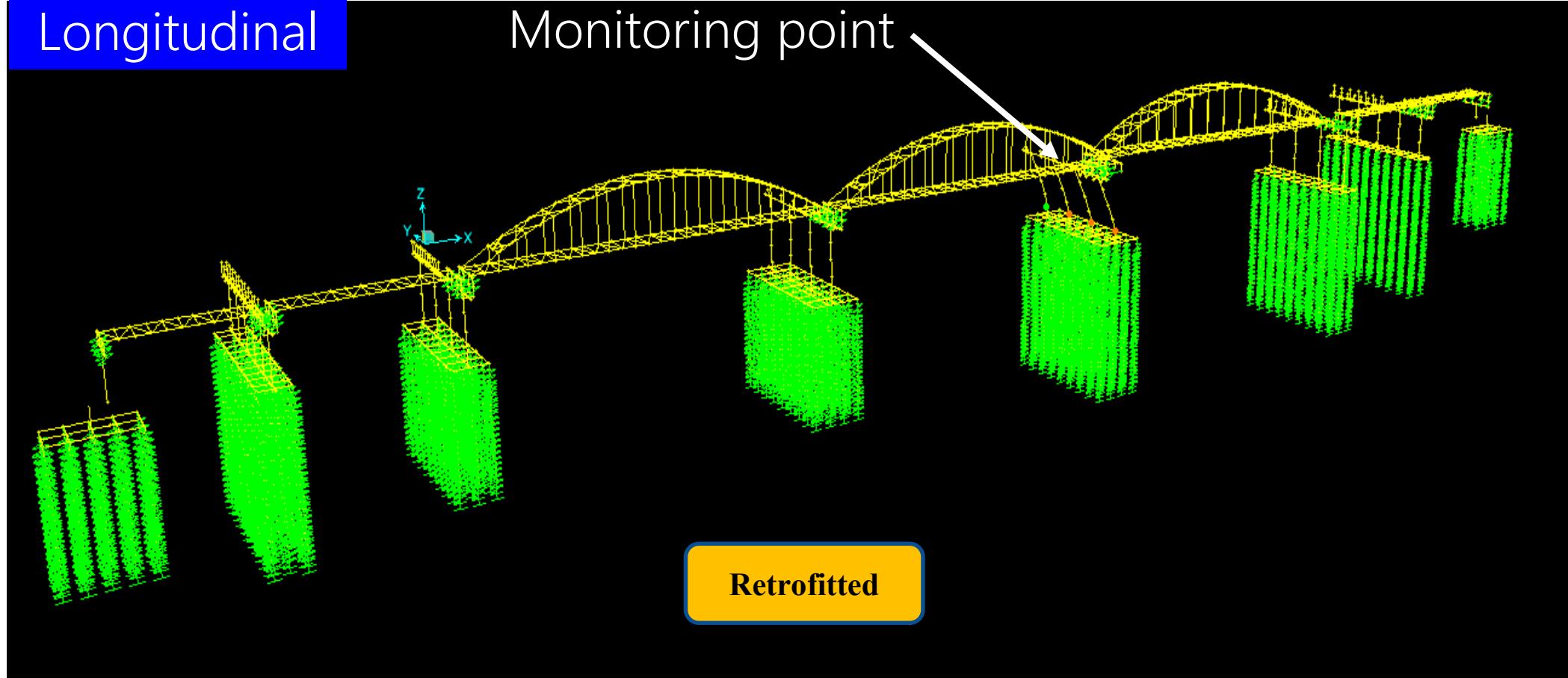
Transverse model: 0.87 sec



Pushover analysis(1/2)

Longitudinal

Monitoring point

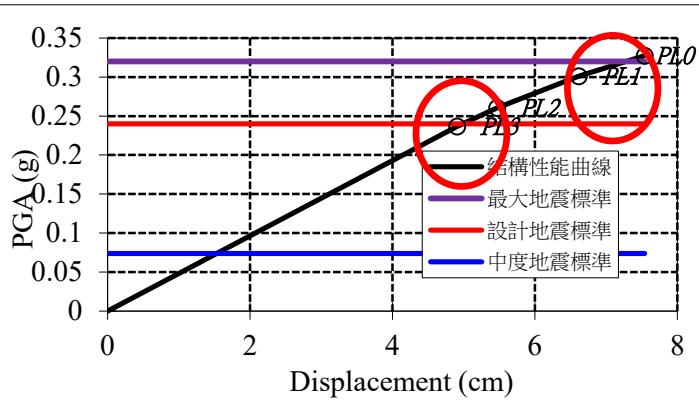
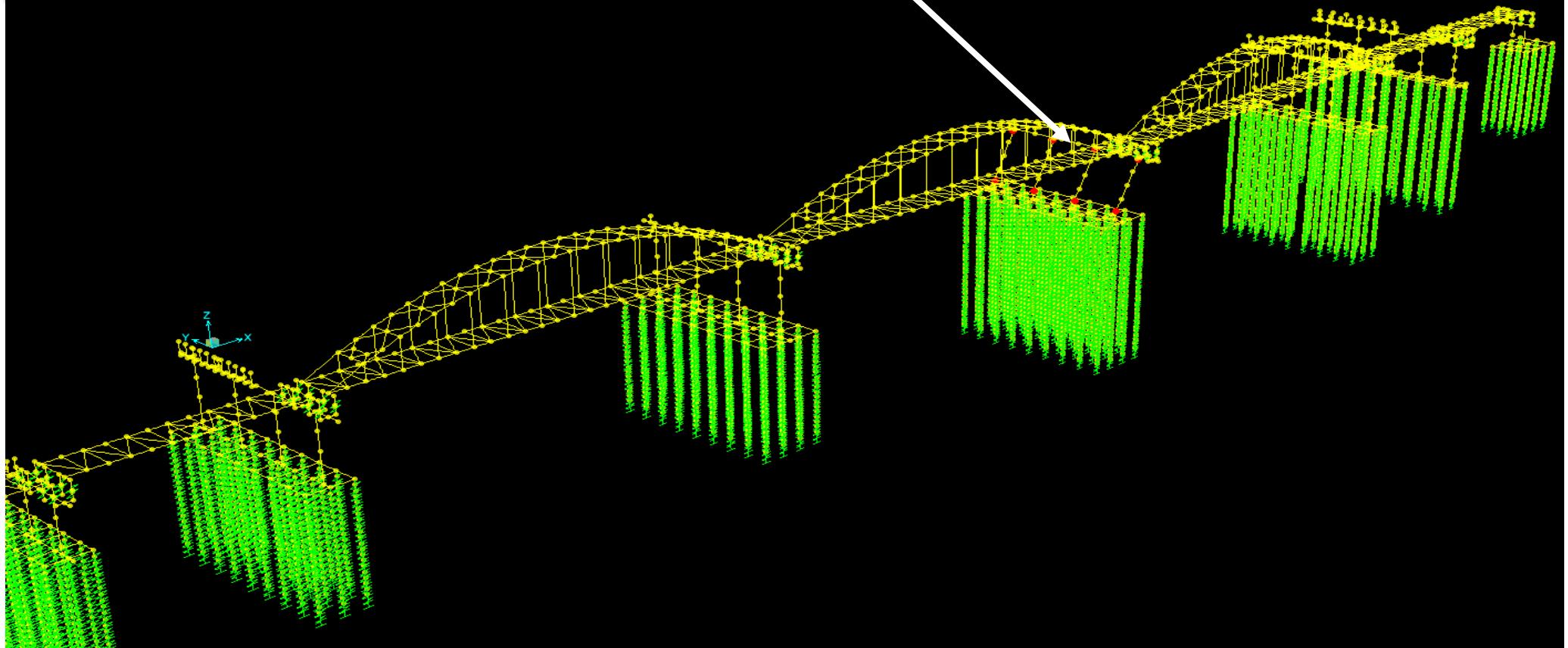


Performance objective		Analysis unit		
Earthquake level	Ground Acc. (g)	performance	PGA (g)	Seismic demand check
Moderate	0.074 g	PL3	0.150	OK
Design	0.240 g	PL1	0.265	OK 34

Pushover analysis(2/2)

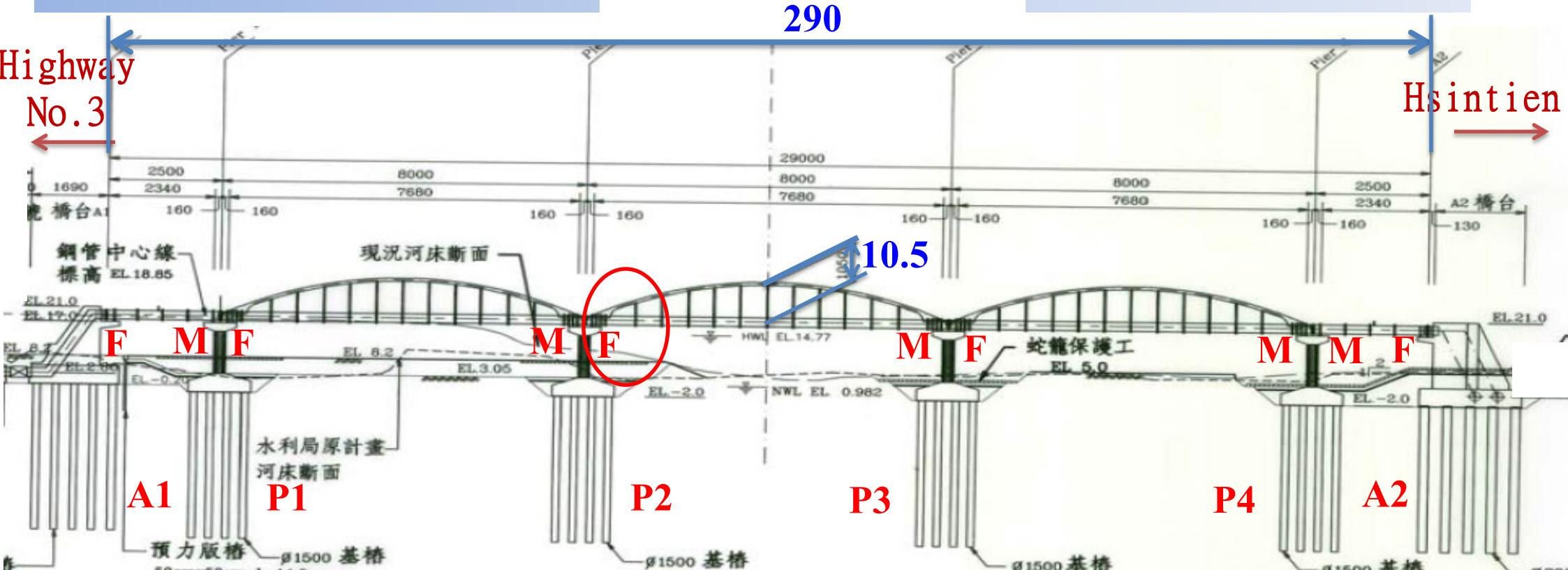
Transverse

Monitoring point



Performance objective		Analysis unit		
Earthquake level	Ground Acc. (g)	performance	PGA (g)	Seismic demand check
Moderate	0.074 g	PL3	0.237	OK
Design	0.240 g	PL1	0.301	OK 35

Water pipe bridge D-structure description

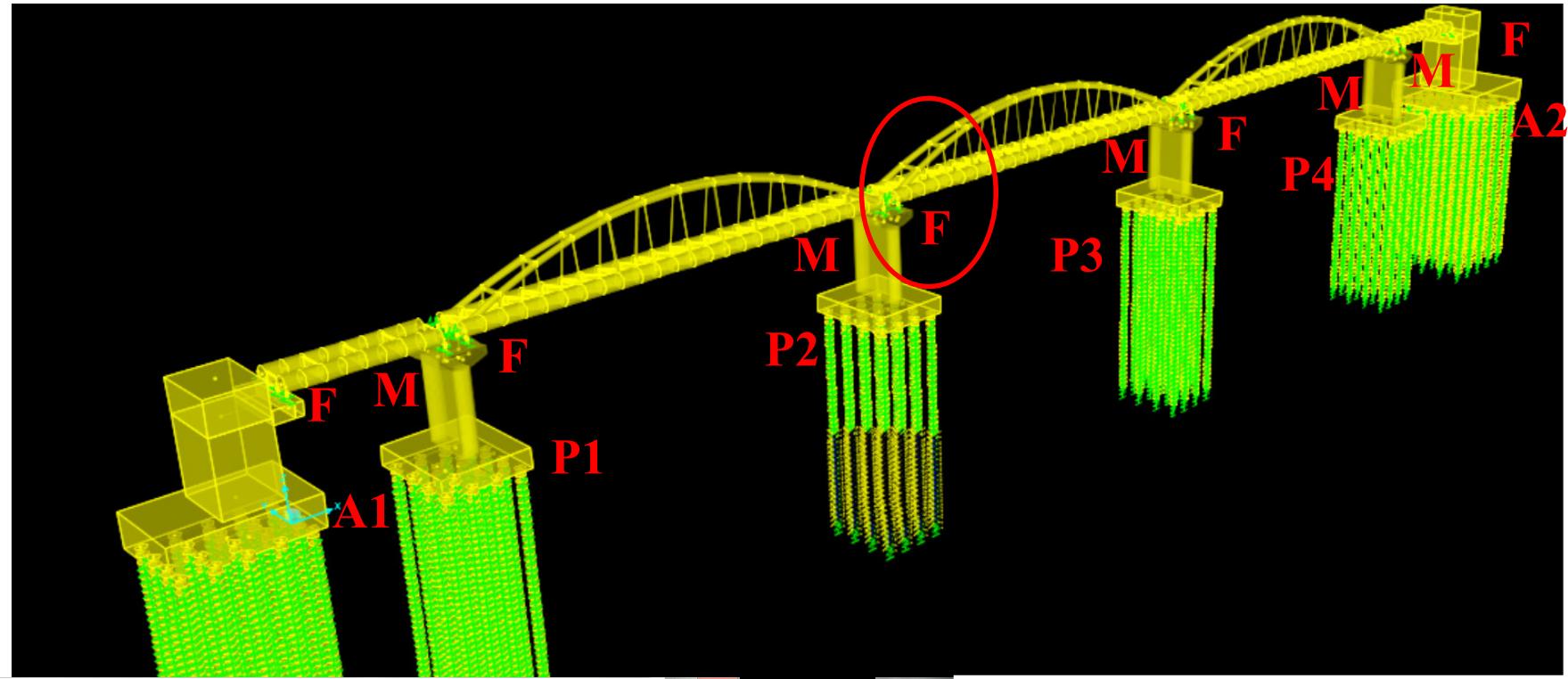


Structure description

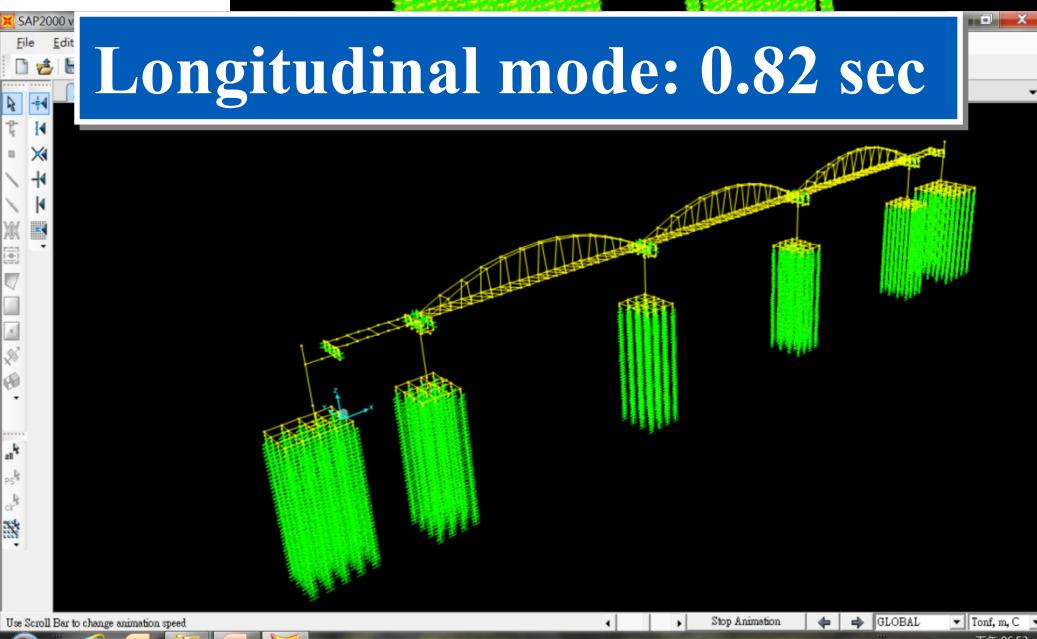
- ❖ multi-span simply supported arch steel bridge; completion year in 1998; Bridge length: 290 m; arch height: 10.5 m
- ❖ Steel pipe diameter: 2400mm;
- ❖ Concrete: $f_c' = 240 \text{ kgf/cm}^2$; steel: A36

New Code

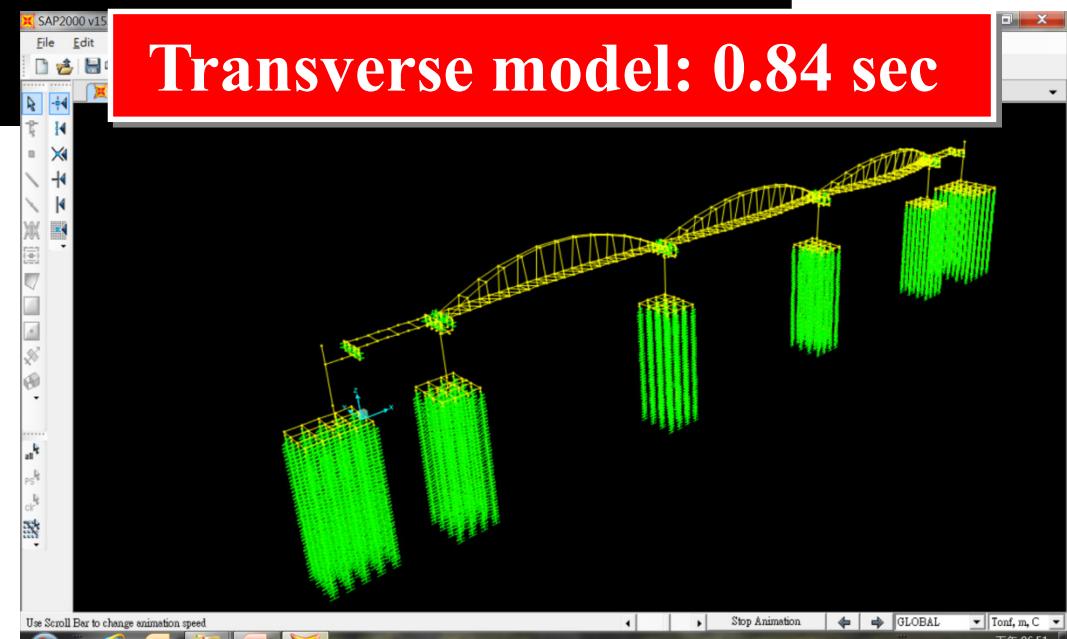
Water pipe bridge D-SAP2000 model and modal analysis



Longitudinal mode: 0.82 sec



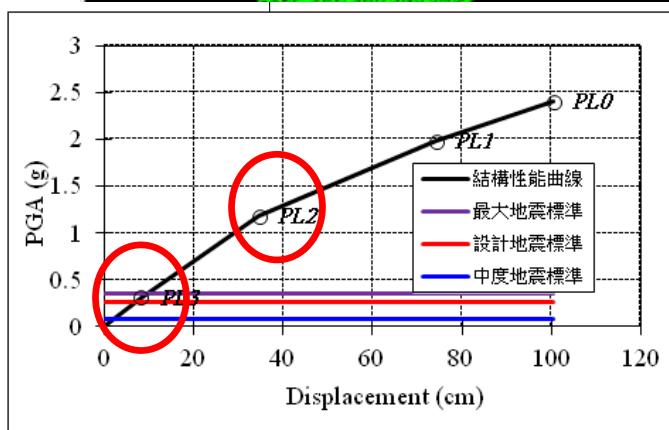
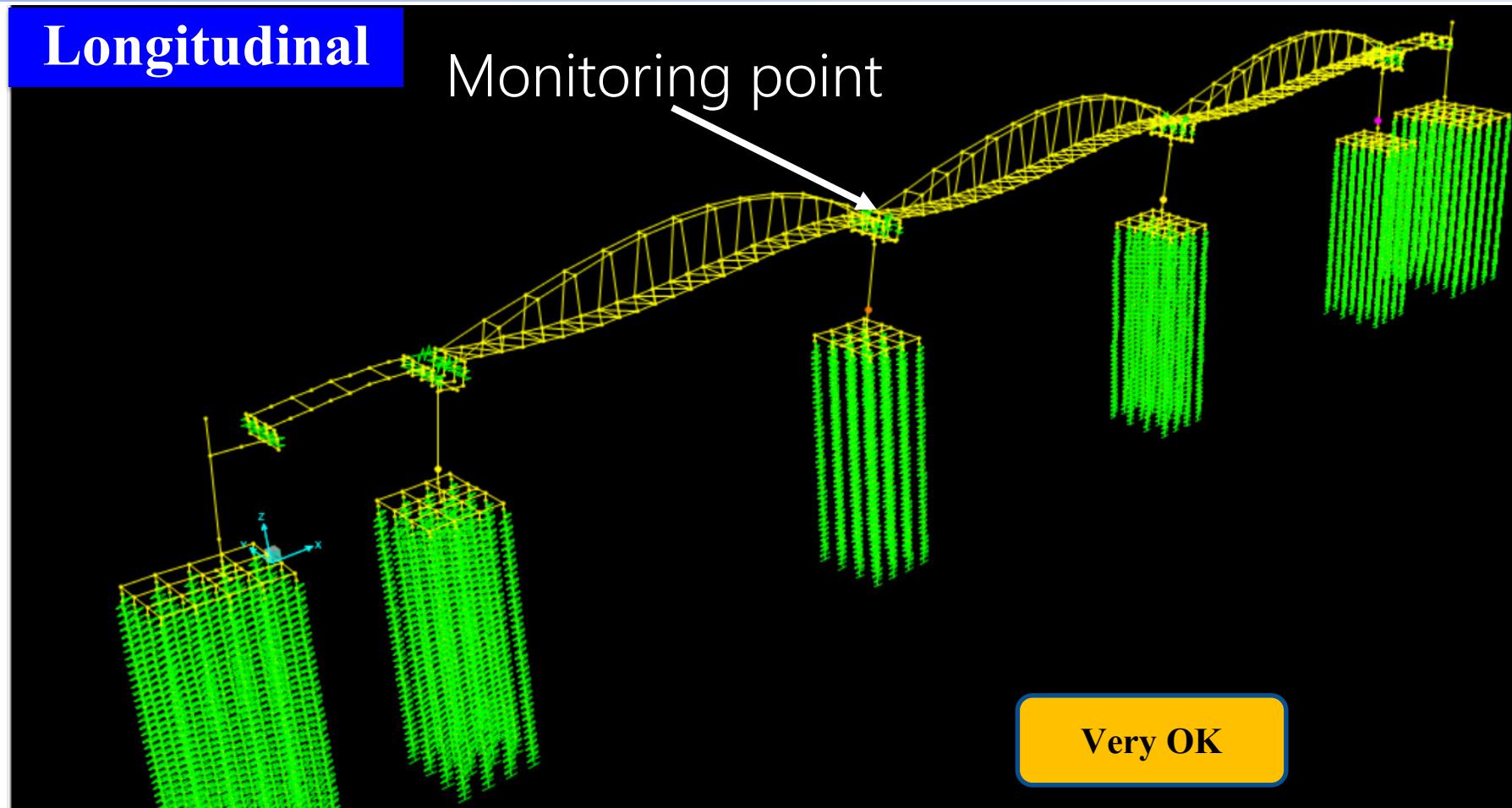
Transverse model: 0.84 sec



Pushover analysis(1/2)

Longitudinal

Monitoring point

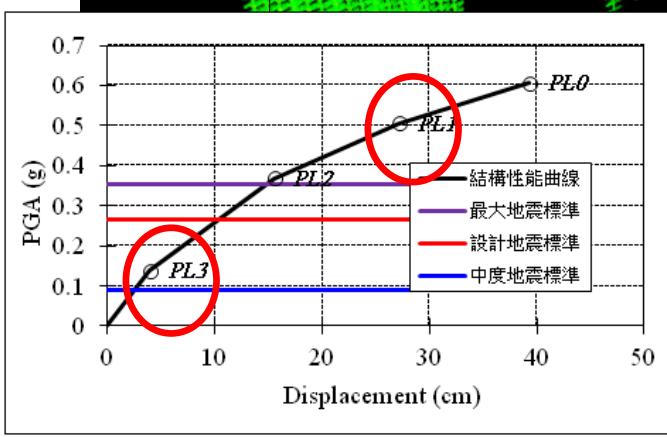
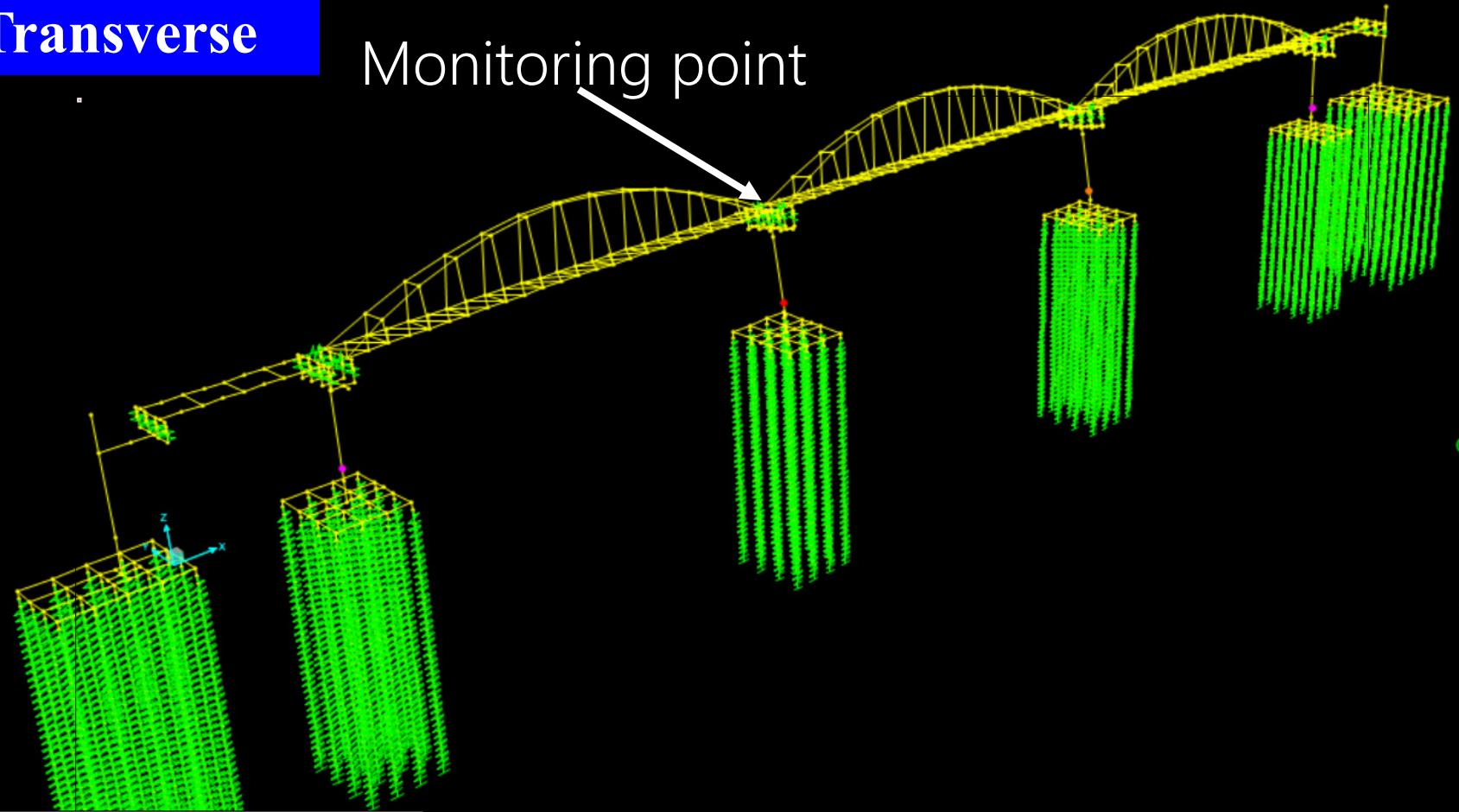


Performance objective		Analysis unit		
Earthquake level	Ground Acc. (g)	perform ance	PGA (g)	Seismic demand check
Moderate	0.074	PL3	0.313	OK
Design	0.240	PL2	1.181	OK 38

Pushover analysis(2/2)

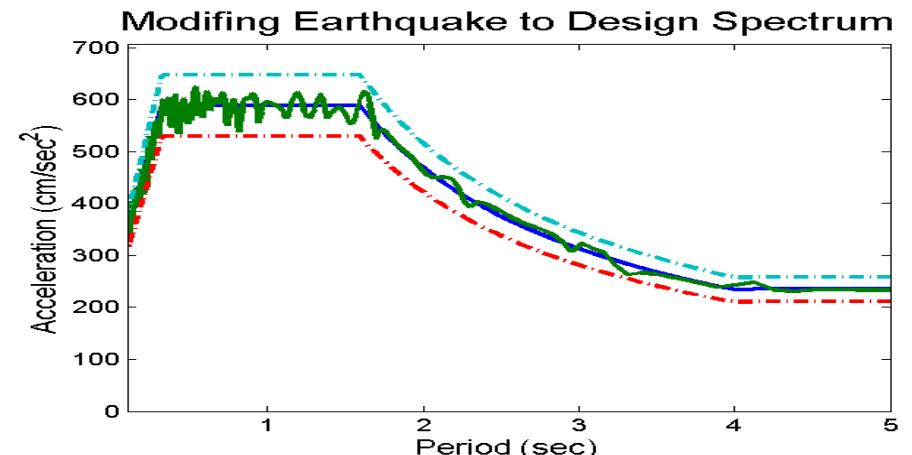
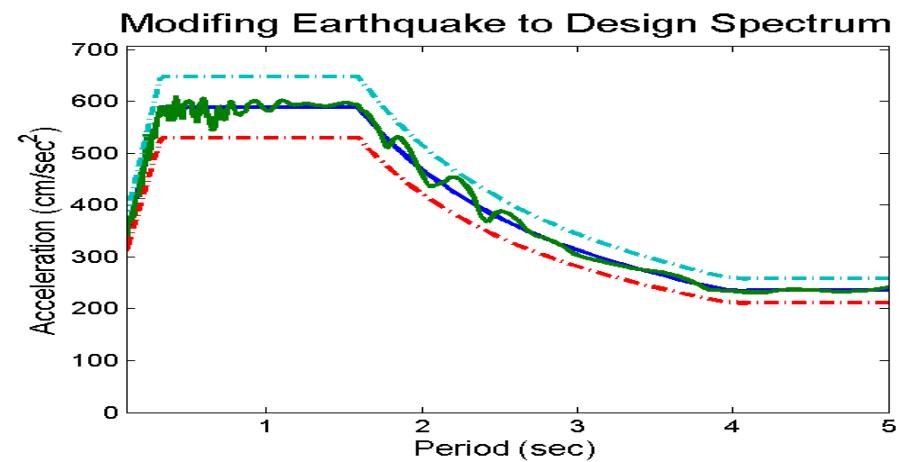
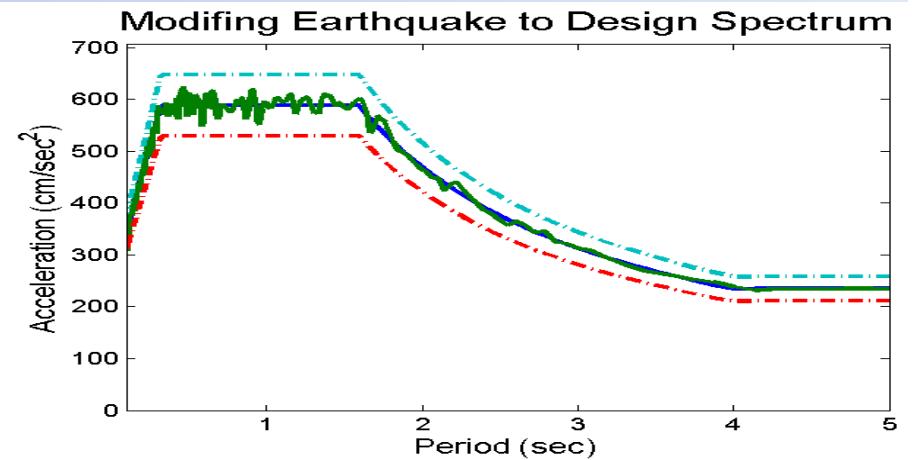
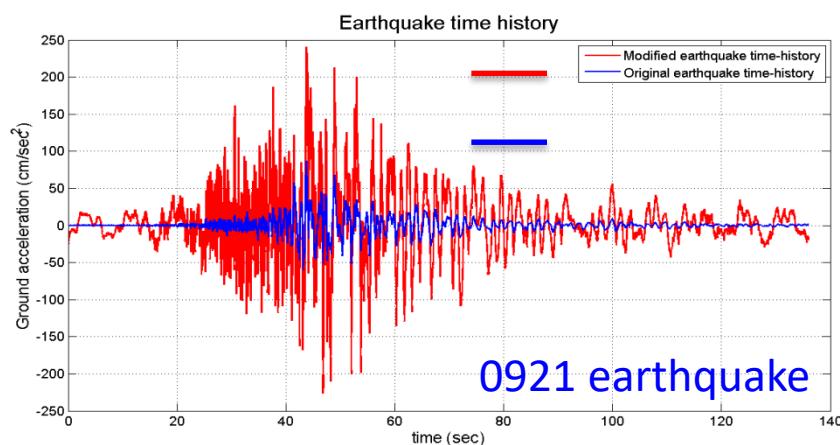
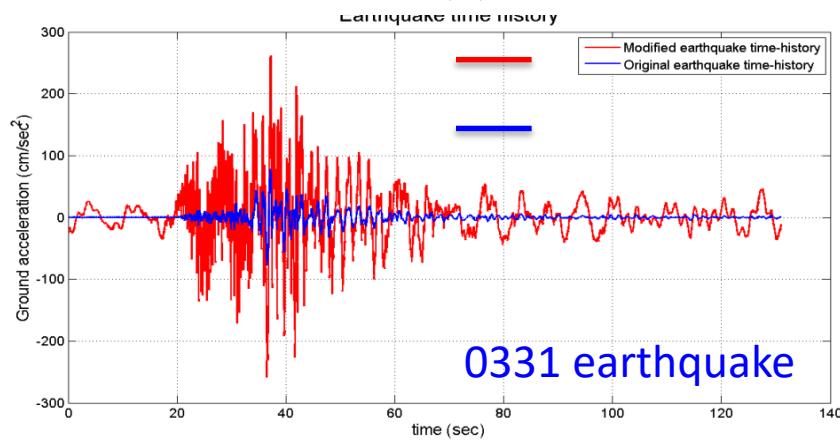
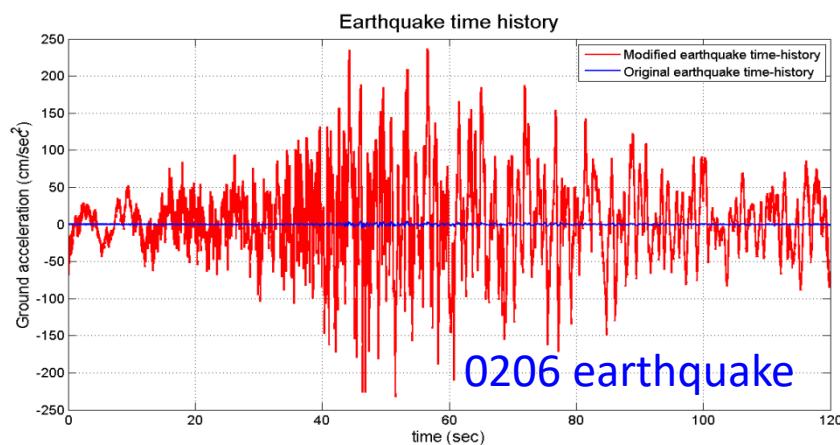
Transverse

Monitoring point



Performance objective		Analysis unit		
Earthquake level	Ground Acc. (g)	performance	PGA (g)	Seismic demand check
Moderate	0.074	PL3	0.138	OK
Design	0.240	PL2	0.369	OK 39

Time history analysis-Artificial time history



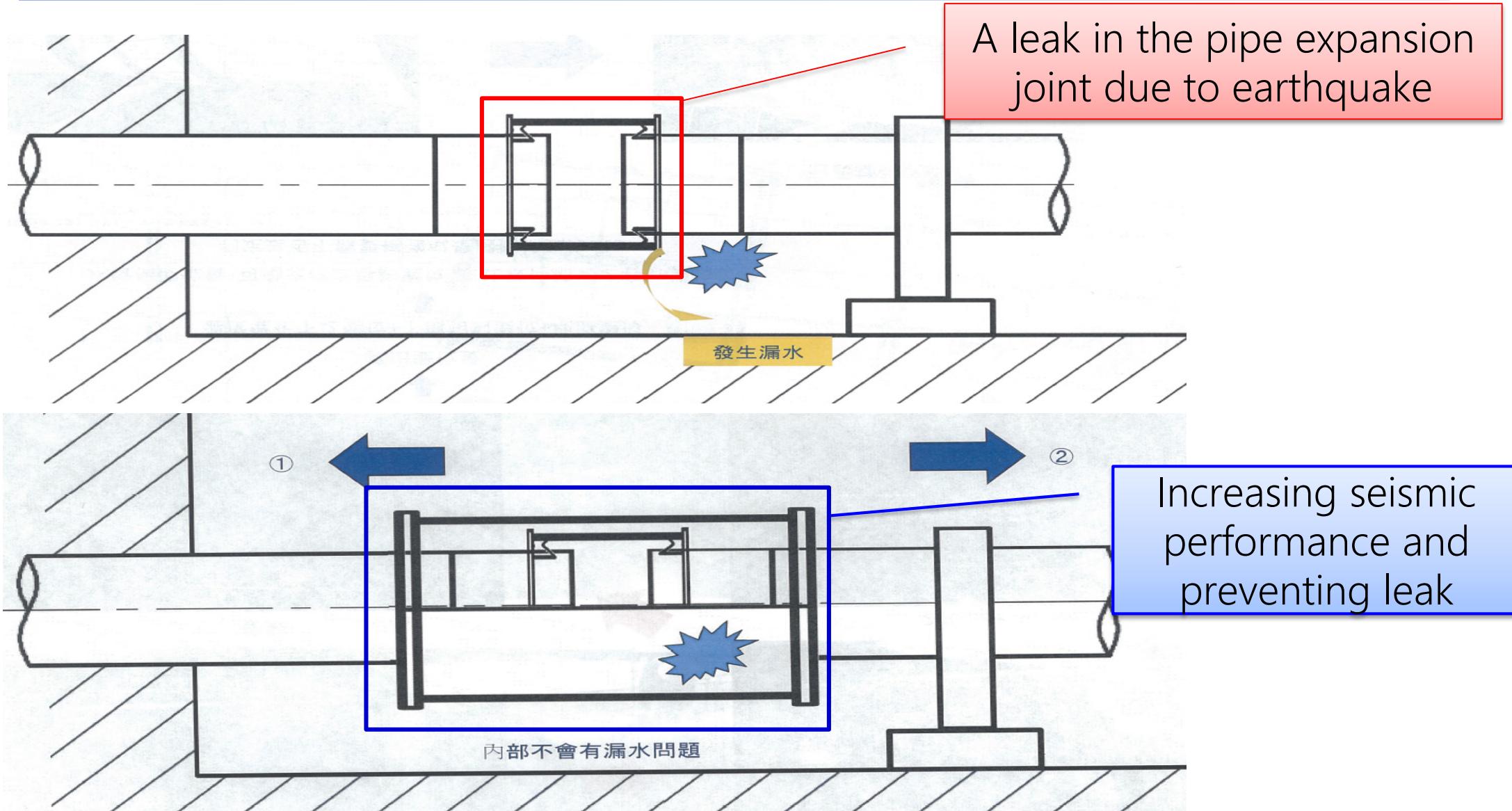
Pipe expansion joint check

Bridge	Earthquake station/record	Axial displacement (Demand)	Bridge	Earthquake station/record	Axial displacement (Demand)
A	TAP007/0206	4.37 cm	B	TAP007/0206	11.83 cm
	TAP008/0331	5.01 cm		TAP008/0331	23.48 cm
	TAP013/0921	4.35 cm		TAP006/0921	15.86 cm
C	TAP029/0206	22.0 cm	D	TAP033/0331	5.71 cm
	TAP028/0331	23.8 cm		TAP053/0401	2.24 cm
	TAP028/0921	21.2 cm		TAP034/0921	4.35 m

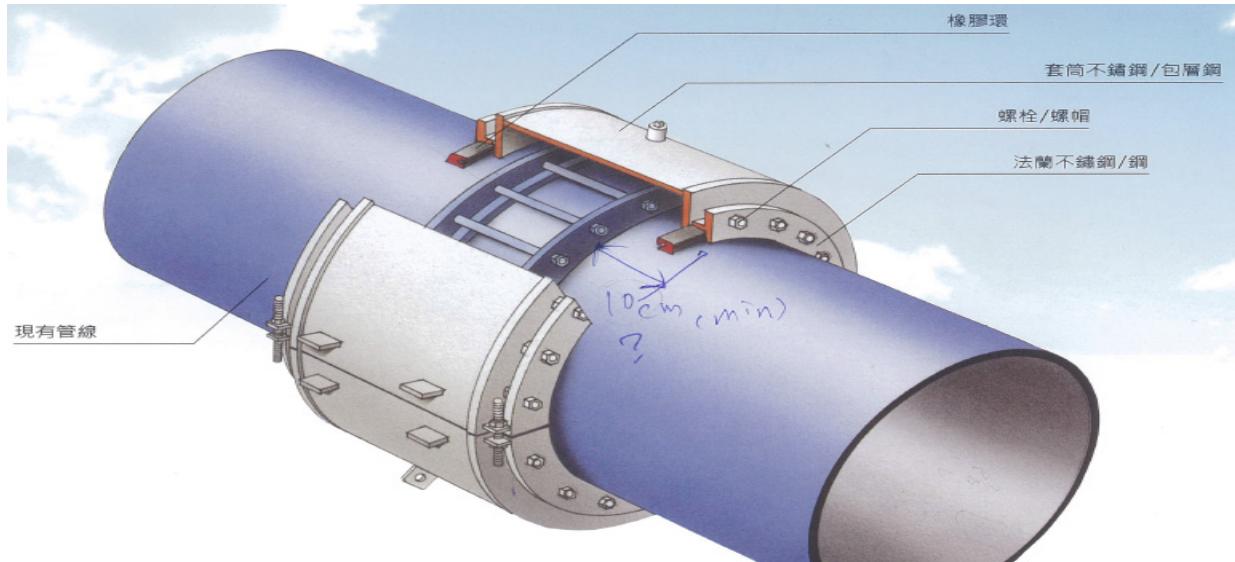
→ Pipe expansion joint demand are larger than flexible capacity

→ Additional Outer flexible expansion joint

Retrofit scheme-Outer flexible expansion joint



Retrofit scheme-Outer flexible expansion joint



Item	Unit	prices(NT)
Flexible expansion joint	lump sum	4,050,000
Tariff (including transport)	--	432,000
Technical support	22 man-day	400,000
Installation and construction	lump sum	480,085

**Small
installation space**

→ **About 5,362,085 (NT)**

Bearing strength check

Bridge		A		B	
Abutment		A1/A2		A1/A2	
Direction		Longitudinal	Transverse	Longitudinal	Transverse
Demand	ton	126.6/--	128.2/128.2	--/--	48/76
Capacity	ton	34.2/--	41.0/41.0	--/--	41.0/41.0
C/D value		0.27/--	0.31 /0.31	--/--	0.85 /0.54
Check		N.G.	N.G.	--/--	N.G.

Bridge		C		D	
Abutment		A1/A2		A1/A2	
Direction		Longitudinal	Transverse	Longitudinal	Transverse
Demand	ton	126.6/--	128.2/128.2	106/89	19.8/35.5
Capacity	ton	34.2/--	41.0/41.0	34.2/34.2	41.0/41.0
C/D value		0.27/--	0.31 /0.31	0.32/0.38	2.0 /1.15
Check		N.G.	N.G.	N.G.	OK

 Bearing seismic demand are larger than Bearing capacity

Bearing Retrofit scheme - steel anti-shock devices

Retrofit scheme	Function Impact	Construction	Costs	Duration	Maintain
Scheme 1	Replace- ment bearing	Acceptable	Acceptable	Acceptable	Acceptable
Scheme 2	Steel anti- shock devices	Excellent	Excellent	Excellent	Excellent



Retrofit scheme 2 is proposed



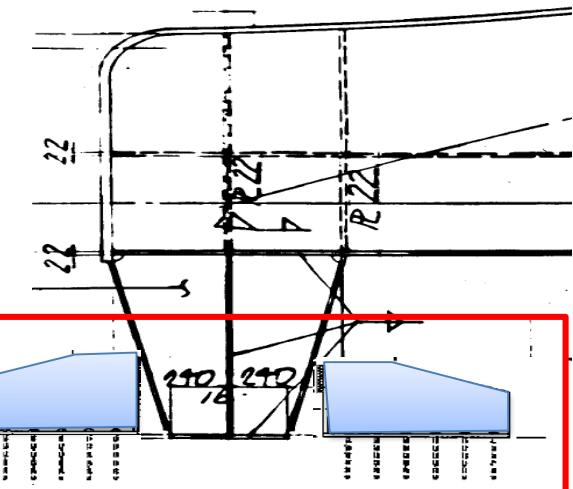
Retrofit scheme - steel anti-shock devices



Steel anti-shock devices

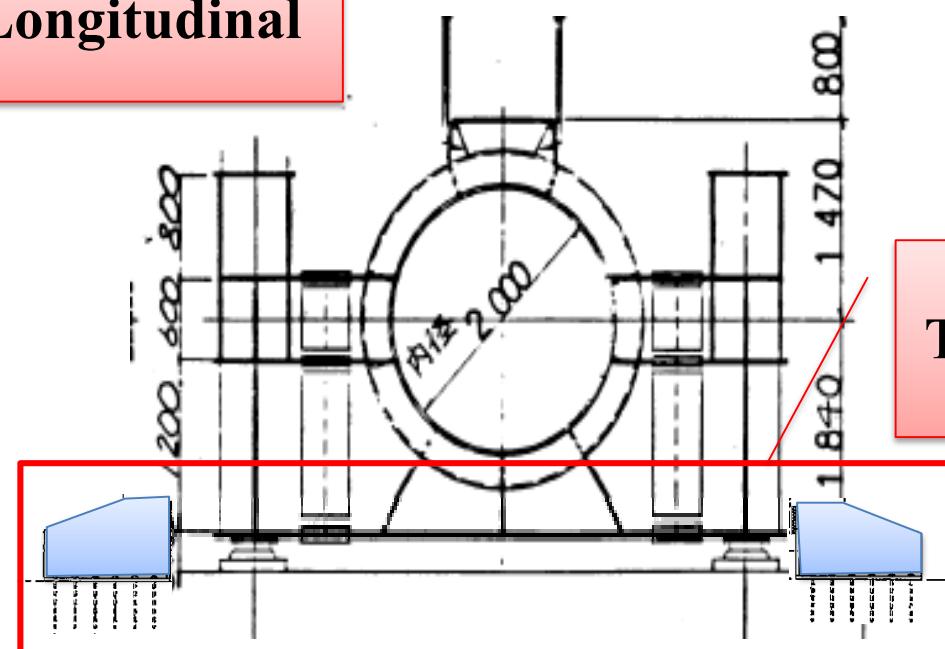


Bearing Side elevation



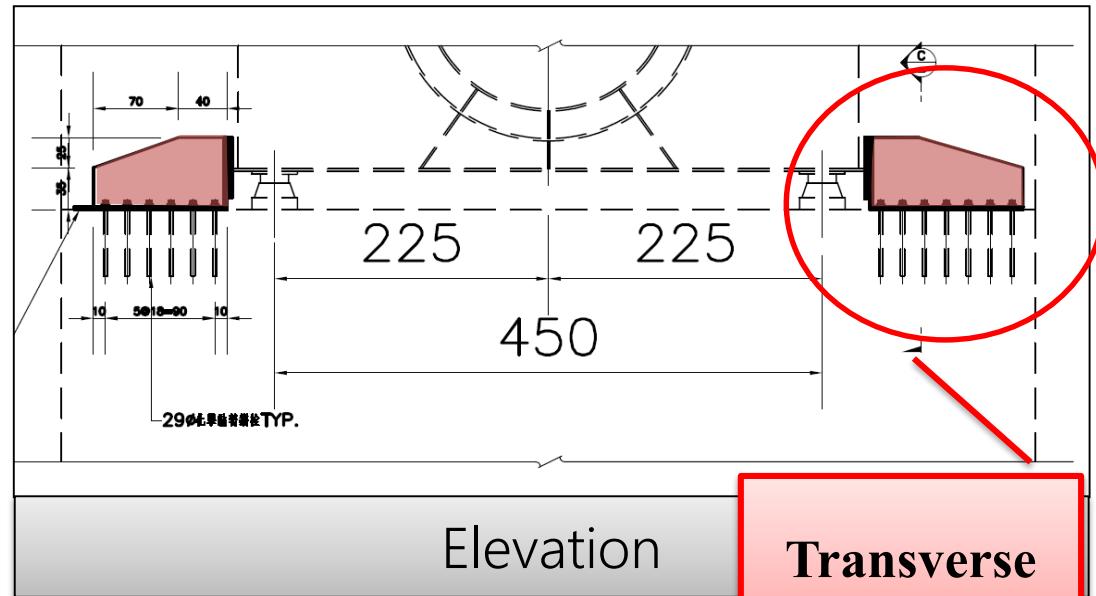
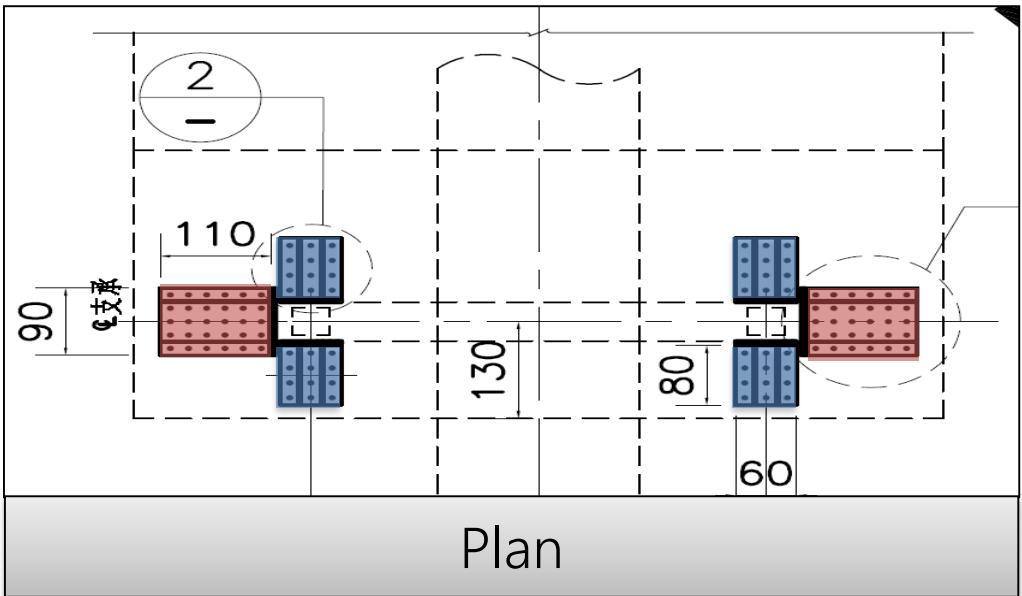
Bearing elevation

Longitudinal

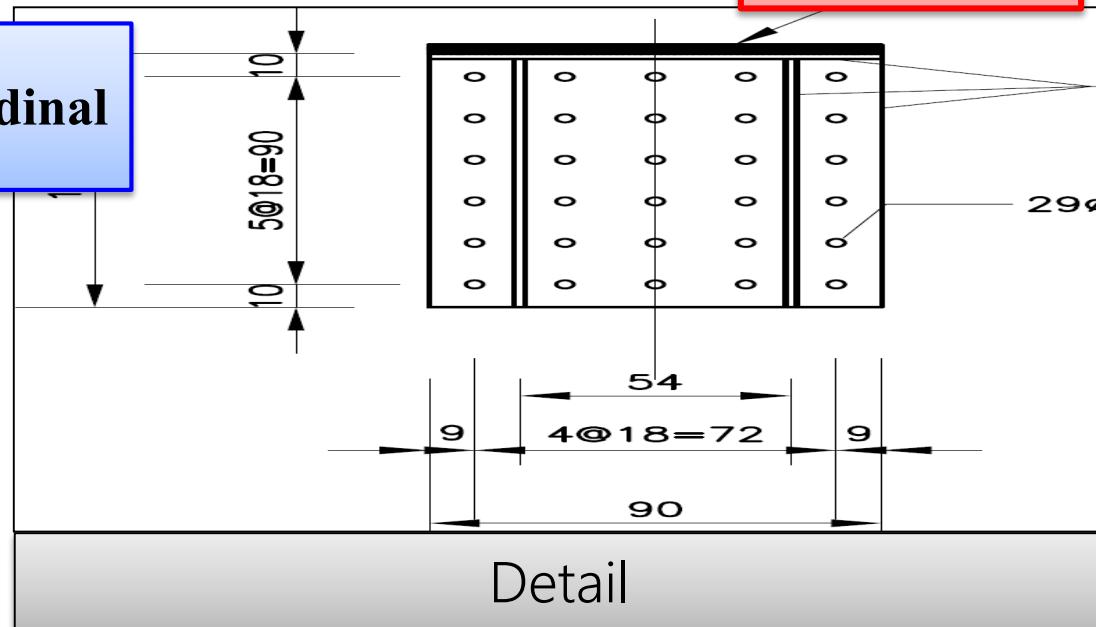
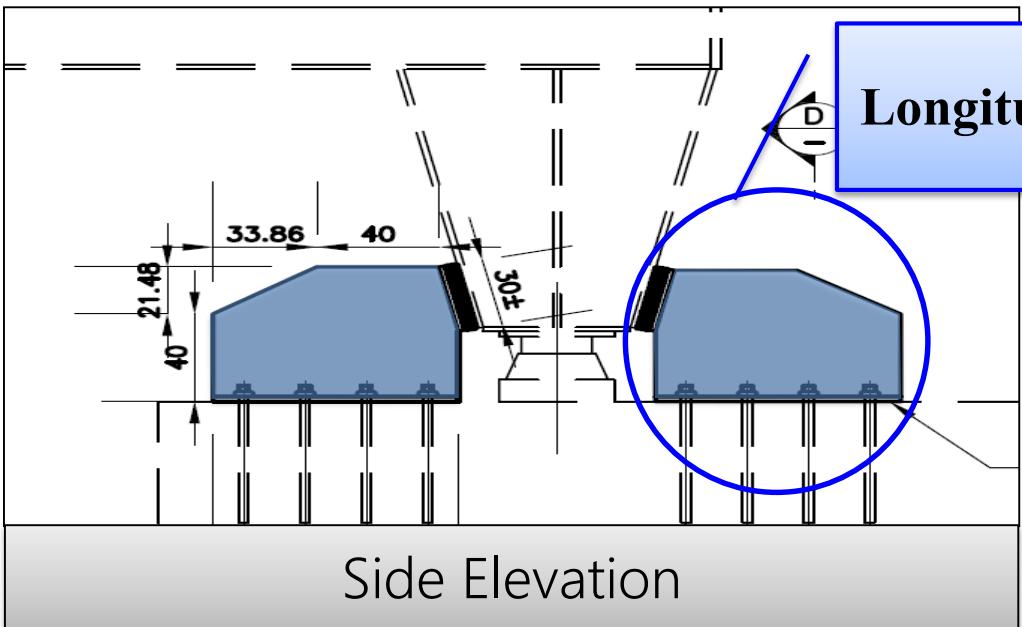


Transverse

Retrofit scheme - steel anti-shock devices



Transverse



04

Conclusion



Conclusion

■ Seismic Evaluation Results & NG parts will be retrofitted

Bridge	Superstructure		1.2 Column Vert. & Hori.	3.Pipe expansion joints	4.Foundation
	Main Girder	3.Bearing			
A Old Code	OK	NG	Abutment ,OK	OK	OK
B Old Code	OK	NG	Just OK	NG	OK
C Old Code	OK	NG	Retrofitted,OK	NG	OK
D New Code	OK	NG	Very OK	OK	Very OK

■ Seismic Retrofit Schemes

❖ The pipe expansion joints:

One is the replacement pipe expansion joint scheme, the other is the additional Outer flexible expansion joint scheme.

❖ The bearings:

One is the replacement bearings scheme, the other is the additional RC or steel anti-shock devices scheme.

Not as safe as
Bridges designed by
New code



Water pipe bridge C

Thanks for your attention!
I'll appreciate your comments!

